STATE OF MINNESOTA
BOARD OF ARCHITECTURE, ENGINEERING, LAND SURVEYING, LANDSCAPE ARCHITECTURE, GEOSCIENCE AND INTERIOR DESIGN

In the matter of Jerry Wayne Anderson
Architect
License Number 13639

STIPULATION AND ORDER

Board File No. 2009-0029

TO: Mr. Jerry Wayne Anderson
Jamb Architects
Post Office Box 310
Forest Lake, Minnesota 55025

The Minnesota Board of Architecture, Engineering, Land Surveying, Landscape Architecture, Geoscience and Interior Design ("Board") is authorized pursuant to Minnesota Statutes section 214.10 (2008) and Minnesota Statutes section 326.111(2008) to review complaints against architects, professional engineers, land surveyors, landscape architects, geoscientists, and certified interior designers, and to take disciplinary action whenever appropriate.

The Board received information concerning Jerry Wayne Anderson ("Respondent"). The Board's Complaint Committee ("Committee") reviewed the information. The parties have agreed that the matter may now be resolved by this Stipulation and Order.

STIPULATION

IT IS HEREBY AGREED by and between Respondent and the Committee as follows:

1. Jurisdiction. The Respondent has held a license to practice architecture from the Board since February 21, 1979. Respondent is subject to the jurisdiction of the Board with respect to the matters referred to in this Stipulation.

2. Facts. This Stipulation is based upon the following facts:

a. Respondent was first licensed as an architect by the State of Minnesota on February 21, 1979.

b. On June 30, 2008, Respondent's Minnesota architect license expired.

c. On October 20, 2008, Respondent's Minnesota architect license was reinstated.

d. As of the date of this Stipulation, Respondent's Minnesota architect license status is current with an expiration date of June 30, 2010.

e. On September 18, 2008, Respondent certified and stamped architectural drawings for the 1st Church of Christ Scientist – church roof re-design & replacement project. A true and correct copy of the complete set of drawings for the 1st Church of Christ Scientist – church roof re-design & replacement project, sheets A-1 and A-2 are available in the Board's office. A partial copy of the sheets A-1 and A-2 drawings, showing the project name, date, and certification are attached as Exhibit A.

f. In a letter dated October 17, 2008, to Lynette DuFresne, the Board's

Investigator, Frank Berg, PE, City of Saint Paul, Minnesota, stated: "The drawings were brought to DSI September 19, 2008, by Mr. Tim Tacheny representing the Church ownership and in pursuit of a building permit. The drawings consisted of architectural sheets A-1 and A-2 and structural sheet S-1. The architectural sheets had been certified by Mr. Anderson the day before (September 18th) and the structural sheet by ...[name of P.E. redacted]...also the day before." A true and correct copy of the October 17, 2008 letter to Lynette DuFresne from Frank Berg, PE, City of Saint Paul, Minnesota, with name of P.E. redacted, is attached as Exhibit B. A true and correct copy of the complete set of drawings, A-1, A-2 and S-1, with name of P.E. redacted, are available in the Board's office. A partial copy of the A-1, A-2 and S-1 drawings, showing the project name, date, and certification, with name of P.E. redacted, are attached as Exhibit A.

g. In the same letter dated October 17, 2008, to Lynette DuFresne, Frank Berg, PE, City of Saint Paul, Minnesota, stated: "I learned that Mr. Anderson's registration had expired as of June 30, 2008. I explained to Mr. Tacheny that this needed to be cleared up by Mr. Anderson, with the Board, before I would be able to finalize my review." A true and correct copy of the October 17, 2008 letter to Lynette DuFresne from Frank Berg, PE, City of Saint Paul, Minnesota, with name of P.E. redacted, is attached as Exhibit B.

h. In a letter to the Board, dated January 22, 2009, Respondent admits: "During the course of finalizing plans for the Church of Christ Scientist, it was

brought to my attention that my Minnesota registration as an Architect had lapsed." A true and correct copy of the January 22, 2009 letter is attached as Exhibit C.

i. In this same letter dated January 22, 2009, Respondent states that he did not receive a renewal notification for his Architect's license. "I did not receive a renewal notification and overlooked the fact that my license was up for renewal. This was definitely my fault for not notifying the Board of change of address for my business at the time of the move, but was entirely a simple oversight." A true and correct copy of the January 22, 2009 letter is attached as Exhibit C.

j. In a letter dated July 8, 2009 to Respondent, Lynette DuFresne, Board Investigator, alleged that Respondent practiced without a license as an Architect and held himself out as an Architect during the time Respondent's Architect license had lapsed. Lynette DuFresne further alleged that the Respondent did not conduct himself properly as an Architect and that the Respondent may have practiced professional engineering without a license by preparing or having drawn the drawings identified as sheet S-1, on 09/18/008, for the 1st Church of Christ Scientist, Church Roof Re-Design & Replacement, of Saint Paul, Minnesota project. A true and correct copy of the letter dated July 8, 2009 letter, with name of P.E. redacted, is attached as Exhibit D.

k. In a letter dated July 24, 2009 from Respondent to Lynette DuFresne, Board Investigator, Respondent admits that he prepared and drew the

plans identified as sheet S-1, dated 09/18/08, for the 1st Church of Christ Scientist - Church Roof Re-design & Replacement project in St. Paul, Minnesota. Respondent states: "[Name of P.E. redacted] did not draw the plans for the 1st Church of Christ Scientist, sheet S-1. The framing plan was discussed with ...[name of P.E. redacted]...and prepared for [name of P.E. redacted]'s review and approval by Jerry W. Anderson. [Name of P.E. redacted]... and I have always agreed that he would do the engineering work and I would draft the plans for his review and approval." "[Name of P.E. redacted]... was to complete this work while I was gone so that we could finalize the project on my return. [Name of P.E. redacted] ran the calculations during my absence, I cannot tell you the exact date, but it was between 9/19/09 and 10/02/09." "[Name of P.E. redacted] did eventually sign and certify the work for the Church." A true and correct copy of the July 24, 2009 letter, with name of P.E. redacted, is attached as Exhibit E.

l. In the same letter dated July 24, 2009 from Respondent to Lynette DuFresne, Board Investigator, Respondent states that the original signature on the sheet S-1 was a sticker given to the Respondent some time ago. Respondent stated: "The original signature on sheet S-1 was a sticker given to me some time ago by [name of P.E. redacted]." A true and correct copy of the July 24, 2009 letter, with name of P.E. redacted, is attached as Exhibit E.

m. In the same letter dated July 24, 2009 from Respondent to Lynette DuFresne, Board Investigator, Respondent states: "I placed [name of P.E.

redacted]'s signature (sticker) on the drawing anticipating that he would be reviewing the final project details and framing plan as well as confirming his load calculations." "We had an understanding that [name of P.E. redacted] would perform all engineering work and that I would draw the plans and details for him. It was not our normal practice to use stickers for signature plates. [Name of P.E. redacted] had given me a few to use for him when timing issues arose or when he might be out of town and deadlines needed to be maintained." A true and correct copy of the July 24, 2009 letter, with name of P.E. redacted, is attached as Exhibit E.

3. Violations. Respondent admits that the facts specified above constitute violations of Minnesota Statutes section 326.02, subdivisions 1 and 2, Minnesota Statutes section 326.111, subdivision 4 (a) (1), (2), (3), and (9) (2008) and Minnesota Rules 1805.0100, and Minnesota Rules 1805.0200, subparts 1 and 4 (C) and (D) (2007) and are sufficient grounds for the action specified below.

4. Enforcement Action. Respondent and the Committee agree that the Board should issue an Order in accordance with the following terms:

a. Reprimand. Respondent is reprimanded for the foregoing conduct.

b. Civil Penalty. Respondent shall pay to the Board a civil penalty of Three Thousand Dollars ($3,000.00). Respondent shall submit a civil penalty of Three Thousand Dollars ($3,000.00) by cashier's check or money order to the Board on or before June 30, 2010.

c. Ethics Course. On or before June 30, 2010, Respondent shall successfully complete, and submit acceptable documentation thereof, a course in professional ethics, four (4) hours, and which is approved in advance by the Complaint Committee. Completion of any courses or activities for the four (4) hours of professional ethics earned on or before June 30, 2010, that are being submitted for the purpose of fulfilling the four (4) hours of professional ethics in this order shall not count toward any continuing education requirements in the 2010-2012 renewal period or beyond.

5. Additional Discipline for Violations of Order. If Respondent violates this Stipulation, Minnesota Statutes Chapter 326 (2008), or Minnesota Rules Chapter 1800 (2007) or Minnesota Rules Chapter 1805 (2007), the Board may impose additional discipline pursuant to the following procedure:

a. The Committee shall schedule a hearing before the Board. At least thirty days prior to the hearing, the Committee shall mail Respondent a notice of the violation alleged by the Committee and of the time and place of the hearing. Within fourteen days after the notice is mailed, Respondent shall submit a written response to the allegations. If Respondent does not submit a timely response to the Board, the allegations may be deemed admitted.

b. At the hearing before the Board, the Complaint Committee and Respondent may submit affidavits made on personal knowledge and argument based on the record in support of their positions. The evidentiary record before the Board shall be limited to such affidavits and this Stipulation and Order. Respondent waives a

hearing before an administrative law judge and waives discovery, cross-examination of adverse witnesses, and other procedures governing administrative hearings or civil trials.

c. At the hearing, the Board will determine whether to impose additional disciplinary action, including additional conditions or limitations on Respondent's practice or suspension or revocation of Respondent's license.

6. Waiver of Respondent's Rights. For the purpose of this Stipulation, Respondent waives all procedures and proceedings before the Board to which Respondent may be entitled under the Minnesota and United States constitutions, statutes, or the rules of the Board, including the right to dispute the allegations against Respondent, to dispute the appropriateness of discipline in a contested case proceeding pursuant to Minnesota Statutes Chapter 14 (2008), and to dispute the civil penalty imposed by this Agreement. Respondent agrees that upon the application of the Committee without notice to or an appearance by Respondent, the Board may issue an Order containing the enforcement action specified in paragraph 4 herein. Respondent waives the right to any judicial review of the Order by appeal, writ of certiorari, or otherwise.

7. Collection. In accordance with Minnesota Statutes section 16D.17 (2008), in the event this order becomes final and Respondent does not comply with the condition in paragraph 4(b) above, Respondent agrees that the Board may file and enforce the unpaid portion of the civil penalty as a judgment without further notice or additional proceedings.

8. Board Rejection of Stipulation and Order. In the event the Board in its discretion does not approve this Stipulation or a lesser remedy than specified herein, this Stipulation shall be null and void and shall not be used for any purpose by either party hereto. If this Stipulation is not approved and a contested case proceeding is initiated pursuant to Minnesota Statutes Chapter 14 (2008), Respondent agrees not to object to the Board's initiation of the proceedings and hearing the case on the basis that the Board has become disqualified due to its review and consideration of this Stipulation and the record.

9. Unrelated Violations. This settlement shall not in any way or manner limit or affect the authority of the Board to proceed against Respondent by initiating a contested case hearing or by other appropriate means on the basis of any act, conduct, or admission of Respondent justifying disciplinary action which occurred before or after the date of this Stipulation and which is not directly related to the specific facts and circumstances set forth herein.

10. Record. The Stipulation, related investigative reports and other documents shall constitute the entire record of the proceedings herein upon which the Order is based. The investigative reports, other documents, or summaries thereof may be filed with the Board with this Stipulation.

11. Data Classification. Under the Minnesota Government Data Practices Act, this Stipulation is classified as public data upon its issuance by the Board, Minnesota Statutes Chapter 13.41, subdivision 5 (2008). All documents in the record shall maintain the data classification to which they are entitled under the Minnesota Government Data

Practices Act, Minnesota Statutes Chapter 13 (2008). They shall not, to the extent they are not already public documents, become public merely because they are referenced herein. A summary of this Order will appear in the Board's newsletter. A summary will also be sent to the national discipline data bank pertaining to the practice of architecture.

12. Entire Agreement. Respondent has read, understood, and agreed to this Stipulation and is freely and voluntarily signing it. The Stipulation contains the entire agreement between the parties hereto relating to the allegations referenced herein. Respondent is not relying on any other agreement or representations of any kind, verbal or otherwise.

13. Counsel. Respondent is aware that he may choose to be represented by legal counsel in this matter. Respondent knowingly waived legal representation.

14. Service. If approved by the Board, a copy of this Stipulation and Order shall be served personally or by first class mail on Respondent. The Order shall be effective and deemed issued when it is signed by the Chair of the Board.

RESPONDENT

Jerry Wayne Anderson

Dated: 6 DECEMBER, 2009

COMPLAINT COMMITTEE

By: Billie Lawton
Billie Lawton, Public Member,
Committee Chair

Dated: 2-05-2010

ORDER

Upon consideration of the foregoing Stipulation and based upon all the files, records and proceedings herein, all terms of the Stipulation are approved and hereby issued as an Order of this Board on this the 5th day of February 2010 ~~2009~~.

MINNESOTA BOARD OF
ARCHITECTURE, ENGINEERING,
LAND SURVEYING, LANDSCAPE
ARCHITECTURE, GEOSCIENCE AND
INTERIOR DESIGN

By: Kristi A. Kubes
Kristine A. Kubes, J.D.
Board Chair

A. Prefinished metal flashing and keepers (copper to match existing)

B. Prefinshed gutters and downspouts (copper to match existing)

C. Existing Brick

D. New Brick (to match existing)

E. New or existing trim – painted (to match existing)

F. Prefinished aluminum louver vents (w/ insect screens)

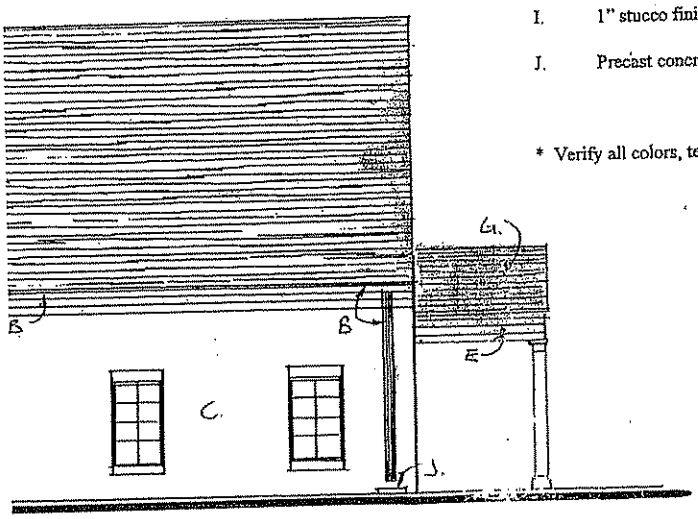
G. 340# fiberglass reinforced asphalt shingles

H. Existing stucco finish

I. 1" stucco finish (texture & color to match existing stucco)

J. Precast concrete splashblocks

* Verify all colors, textures, finishes and material selections with Owner prior to application



FIRST SUBMITTAL

WEST ELEVATION
1/8" = 1'-0"

EXHIBIT A

Jamb architec
P.O. Box 310 Forest Lake, Minnesota
(612) 67

I hereby certify that this plan was prepared by me or under my direct supervision and that I am a duly registered Architect under the laws of the State of Date 9/18/08 Reg. No. 13632

1st CHURCH of CHRIST SCIENTIST

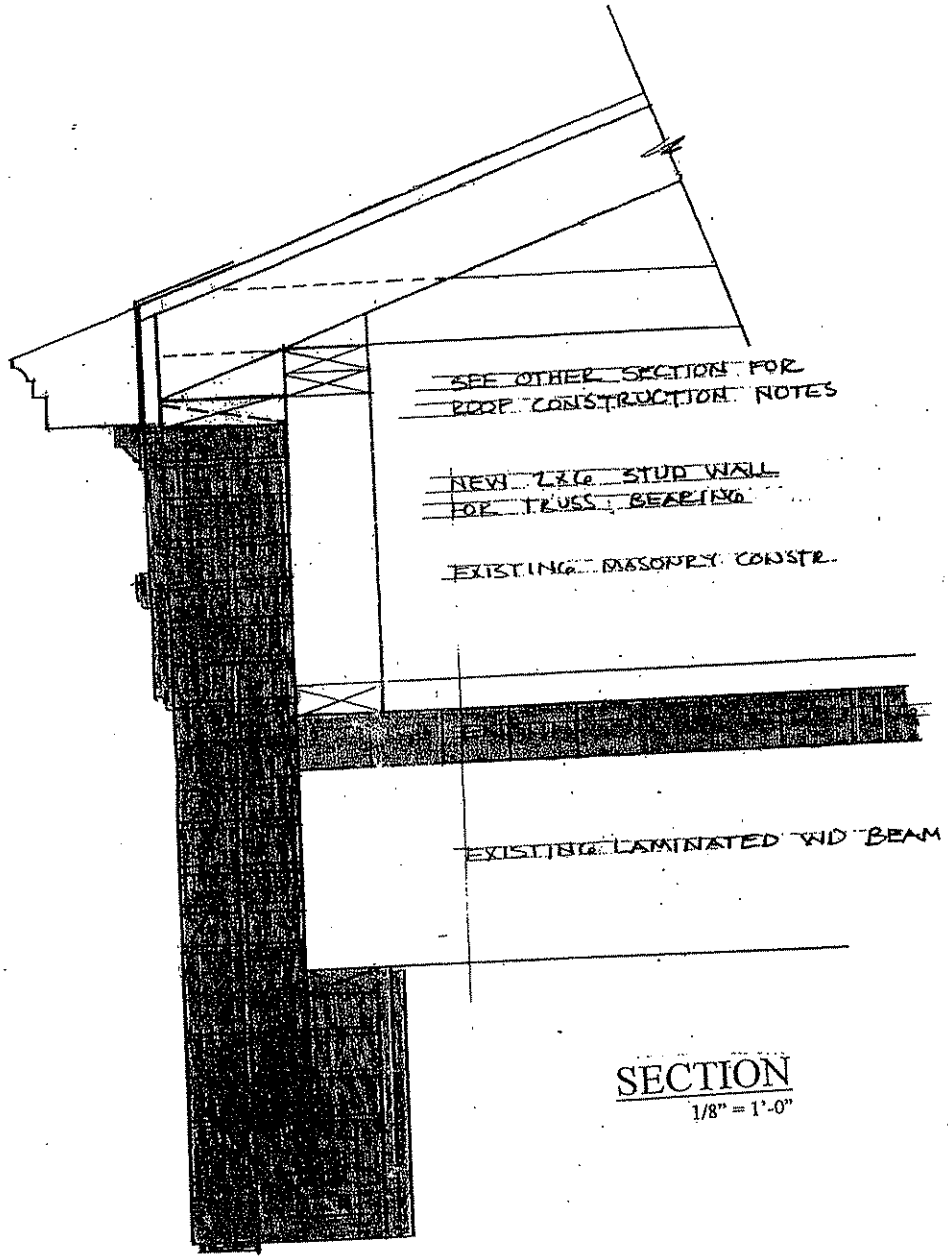
CHURCH ROOF RE-DESIGN & REPLACEMENT

Project No 0810

Date: 9/18/08

Revisions:

Sheet No. A-1



SECTION
1/8" = 1'-0"

Jamb area
P.O. Box 310 Forest Lake, MN

I hereby certify that this plan was prepared by me or under my direct supervision and that I am a duly Registered Architect under the laws of the State of Minnesota.
9/18/08 Reg. No. 13687

1st CHURCH of CHRIST SCIENTIST

CHURCH ROOF RE-DESIGN & REPLACEMENT
MINNESOTA

Project No. 0810
Date: 9/18/08
Revisions:

Sheet No. A-2

ISED FOR DESIGN:
onal Building Code w/ State of Minnesota Appendix

s shall bear on natural undisturbed soil or on compacted granular fill.
ons for subcutting soils in the footing areas referred to in the soil
ed above shall be strictly followed.
ttom of the footings as detailed is of questionable bearing value, the
neers office shall be notified at once.
vation changes shall be stepped at a ratio of 1 (vert) to 2 (horiz).
vertical step shall be 1'-4" unless noted otherwise.
ll footings shall have a minimum soil cover of 3'-8" measured from
e footing unless otherwise noted.
for anticipated settlement values. The structural engineer shall verify
ent criteria will not be detrimental to the building or it's operation.

s, corridors and stairs	100 psf
	*10 psf + 50 psf
	125 psf
	***40 psf
	90 mph
	Material Self Weights

ight if heavier
for sliding snow

	Type Mix	Location
	Std. Weight	Interior slabs and walls
	Std. Weight	Exterior slabs and walls
	Air Entrained	
	Std. Weight	Footings
	Std. Weight	Interior Topping
	f'm = 1,500 psi (28 days)	
	Fy = 60,000 psi	ASTM A615
	Fy = 36,000 psi	ASTM A36
ng	Fy = 46,000 psi	ASTM A500, Grd. B
	Fy = 36,000 psi	ASTM A53, Grd. B
	Fy = 36,000 psi	ASTM A36
	Fu = 120,000 psi	ASTM A325
	Fu = 60,000 psi	ASTM A307
	Fy = 70,000 psi	
bric		ASTM A185

GE FOR REINFORCEMENT:
3"
2"
Center of Slab
Center of Wall

GE FOR REINFORCEMENT:
Center Line (unless otherwise noted)
3" to ties

L:
steel contractor shall fabricate all reinforcement and furnish all
s, spacer bars and supports necessary to secure the reinforcement
erwise on the plans and details.
cement shall be placed according to the CRSI "Recommended
ing Reinforcing Bars".
l tension lap splices for cast-in-place concrete shall be 36 bar
um, unless noted otherwise.
ns for reinforced masonry shall be 40 bar diameters minimum,
rwise.
rcing steel in footings and concrete walls shall be continuous

should be one mesh plus two inches at splices.
einforcing bars each side around openings in concrete walls and
extend 24" beyond the corners of the openings. Also provide 2 -
at each corner of the opening.
shall not be welded without the approval of the structural engineer.
reinforcing steel shall be performed by welders specifically certified
eel. Prior to welding the "carbon equivalent" (CE) of steel shall be
nforcing steel whose "CE" can't be identified or whose "CE" exceeds
e welded. Except for reinforcing steel conforming to ASTM A-706
shall be preheated as shown in table 1.RGA 3-77. In addition, steel
een 0.66% and 0.75% shall be welded only when prior qualification
table weldability.

NRY:
ing masonry units shall conform to ASTM C90 Grade N Type I, in
quirements of the quality control standards of the Concrete/Masonry
imum required compressive strength of block units shall be 2800 psi
a)
all have been cured for not less than 28 days when placed in the

tures will not be permitted in the grout or mortar unless substantiat-
ted to and approved by the structural engineer or the architect.
lly reinforced masonry walls and bond beams shall have fc = 3,000
d shall have a slump of 9" to 10".
s shall have horizontal reinforcing consisting of galvanized standard
r-O-Wall" or equal. All reinforcing shall be located every other course
rwise.
shall match the size and number of footing reinforcing unless noted
into footing 12" minimum and lap 30 diameters with main steel.
red for bearing walls below grade. Type "N" mortar is required for
ve grade. Mortar shall be of a portland cement type mix.
Plan for location and detail of vertical control joints.
g for control joints in masonry walls shall not exceed 24'-0" o.c.
aring on masonry shall have two cores minimum filled with grout
bearing point, except as noted on the plans.
smaller use one #5 continuous. Bond beams 10" or larger use 2 -
less noted otherwise.
hall be performed in accordance with ASTM E447-84. Prepare 1
or to the start of construction. Test 2 of these a 7 days and the
days. Prepare 1 additional set of three prisms for every 5000 sf
est 1 of these at 7 days and the remaining 2 at 28 days. Refer to
ns" in the structural notes and to the project specifications for
tion.

OOD TRUSSES:
wood trusses shall be in accordance with manufacturer's written recommendations
all have bridging and bracing in accordance with manufacturer's requirements.
all be designed for a top chord live load as noted under Design Live Loads plus
in accordance with applicable building codes (minimum top chord dead load shall
num bottom chord dead load shall be 10 psf.).
tion limitations – roof trusses less than L/360.
e shop drawings for approval, showing the erection plan, all bearing conditions,
. Calculations certified by a Professional Engineer will be required for all wood
ions and bearing attachments/components. (Submit certified plans and calcula-
l building official).
olier shall detail end bearing of trusses so as not to exceed perpendicular to grain
plates that support the trusses.

proved by the Engineer.
d. All nailing shall conform to the nailing schedule unless noted otherwise.
e. All sills and plates resting on concrete or masonry, which is in contact with the earth or resting on foundations shall be pressure treated Southern Pine No. 2.
f. All bolt heads and nuts bearing on wood shall have standard cut washers. All bolt holes in wood shall be drilled 1/32" larger in diameter than nominal bolt diameters.
g. Bolts in wood shall not be less than 7 diameters from the end & 4 diameters from edge of member.
h. All framing anchors, post caps, bases, hangers, straps, etc., shall be as manufactured by "Simpson" Company or approved equal.
i. Top plates of all wood stud walls to be 2 – 2x (same width as studs), lap 48" (min.) with not less than 6-16d nails at each lap and not more than 16" between nails. Splice at studs only.
j. Moisture content of wood at time of placing shall not exceed 19%.
k. All member sizes given on the drawings are nominal sizes.
l. Spacing of bridging for joists shall not exceed 8'-0".
m. Wood lintels and headers shall have a full 3" length of bearing at each end (min.).
n. Double all joists under parallel partitions.
o. All beams and joists not bearing on supporting members shall be framed with "Simpson HHUS" joist hangers or equal. Use type as required for the application shown on the drawings.
p. Wood joists shall bear the full width of supporting members (stud walls, beams, etc.) unless otherwise indicated.
q. Wood beams made of 2 or more 2x's shall be nailed together with 3 rows of 16d nails at 12" o.c. For a 3 piece member, install the specified nailing on each side.
r. Unless noted, 4" wide stud walls are to have 2 x 4 studs at 16" o.c.
s. Unless noted, exterior walls are to be 2 x 6 studs at 16" o.c.
t. Sill plates to be bolted to foundation walls with 5/8" round bolts at 6'-0" o.c. maximum, bolts to Extend 15" minimum into grouted masonry. Each sill plate to have a minimum of 2 bolts with one bolt located within 12" of each end of each plate.

11. NAILING SCHEDULE:
a. joists or rafters to sides of studs
8" joist or less 3 -16d
for each additional 4" in depth of joist 1 – 16d
b. bridging to joists, toenail each end 2 – 8d
blocking between joists or rafters to joists or rafters – toenail each side, each end 2 – 10d
blocking between studs each end 2 – 10d toenails or 2 – 16d
c. 1" x 6" subfloor to each joist, face nail 2 – 8d
d. wider than 1" x 6" to each joist, face nail 3 – 8d
e. 2" subfloor to joist or girder, blind and face nail 2 – 16d
f. Sole plate to joist or blocking, face nail 16d @ 16" o.c.
g. Top plate to stud, end nail 2 – 16d
h. Stud to sole plate 4 – 10d toenail, or 2 – 16d end nail
i. Double top plates, face nail 16d @ 16" o.c.
j. Continuous header, two pieces 16d @ 12" o.c. along each edge
k. Ceiling joists to plate, toenail 3 – 8d
l. Continuous header to studs, toenail 4 – 8d
m. Ceiling joists, laps over partitions, face nail 3 – 16d
n. Ceiling joists to parallel rafters, face nail 3 – 16d
o. Joists or rafters at all bearing, toenail each side 2 -10d
p. 1" brace to each stud and plate, face nail 2- 8d
q. 1" x 8" sheathing or less to each bearing, face nail 2 – 8d
r. wider than 1" x 8" sheathing to each bearing, face nail 3- 8d
s. Built-up corner studs 16d @ 24" o.c.
t. Double studs 10d @ 12" o.c. direct
u. Rim joists 6" or less 16d (2/ joist) end nail
v. Rim joists 6" or more 16d (3/ joist) end nail
w. Diagonal bracing (to stud & plate) 16d (3/ joist) end nail

12. BACKFILLING:
a. No backfilling and compacting of earth shall be permitted against foundation walls until supporting slabs have been poured and have reached 75% of their design strength or unless adequate bracing submitted for review has been approved.
b. Both sides of foundation walls shall be backfilled simultaneously so as to prevent overturning or lateral movement of walls.
c. All grade beams shall be adequately braced to prevent lateral movement during backfilling and compaction.

13. CONSTRUCTION AND CONTROL JOINTS:
a. Construction joints shall be made as detailed on the drawings.
b. Maximum spacing for control joints shall be 15'-0".
c. A 15'-0" maximum spacing of control joints may not insure complete control of shrinkage cracks. A closer spacing may be used by request of the contractor if more complete shrinkage crack control is desired. Contractor to verify with Owner.

14. EXPANSION BOLTS:
a. All expansion bolts shall be KWIK bolts or REDHEAD as noted on the plans. Minimum embedment unless otherwise noted shall be 4" for 1/2" diameter bolts, and 5" for 5/8" and 3/4" diameter bolts.

15. CONSTRUCTION PROCEDURES:
a. The structure shall be adequately braced and shored against wind and construction loads during erection. Structural members are designed for "in-place" loads only.
b. Comply with all applicable city, state and federal laws, including Occupational Safety and Health Administration Act (OSHA) and regulations adopted pursuant thereto.
c. The contract structural drawings and specifications represent the finished structure. Unless otherwise noted, they do not indicate the means or method of construction. Provide all measures necessary to protect the structure, workmen, or other persons during construction. Such measures shall include, but are not limited to, bracing, shoring for construction equipment, shoring for earth loads, forms, scaffolding, planking, safety nets, support and bracing for cranes and gin poles, etc.
d. Engage properly qualified persons to determine where and how temporary precautionary measures shall be used and inspect same in the field. Observation visits to the jobsite by the ENGINEER'S representative shall not induce inspection of the above items.
e. Supervise and direct the work so as to maintain sole responsibility for all construction means, methods, techniques, sequences, and procedures. As a part of this responsibility, retain the services of a licensed structural engineer to design and supervise any scaffolding for workmen, and all shoring of forms and elements of the construction.

16. COORDINATION WITH ARCHITECTURAL DRAWINGS:
a. The contractors shall verify all dimensions and elevations with the architectural drawings. Where discrepancies occur, it is the contractors responsibility to notify the Architect prior to proceeding with construction.

17. NEW WORK IN CONJUNCTION WITH EXISTING CONSTRUCTION:
a. The contractors shall verify, by field check, all sizes, dimensions, elevations, locations, etc. of elements of the existing construction which are relative to the new construction.
b. All dimensions involving new work tying into or governed by existing construction shall be field checked by the contractors and furnished to the subcontractors prior to fabrication of any work. The verified dimensions shall appear and be noted as such on the first shop drawings submitted.
c. Cutting of existing structural steel is prohibited with out approval of the engineer.

18. SHOP DRAWINGS:
a. Shop drawings, unless otherwise noted, shall be submitted on reproducible transparencies with one print for review prior to fabrication. Send reproducible transparencies and print in a mailing tube. Transparencies which will not produce a legible print shall be returned for resubmittal.
b. Shop drawings are to be prepared under the supervision of a registered professional engineer. They are to include complete details and schedules for fabrication and assembly of structural members, procedures and diagrams.
c. Fabricators shall draw their own erection plans. Copying the structural plans and using them as erection plans is not acceptable.
d. Prior to submittal, the contractors shall review the shop drawings and make any corrections required. The contractors shall stamp and sign the drawings as evidence that they have reviewed them.
e. Shop drawings shall be submitted for all structural components.
f. Turn around time for shop drawings shall be two weeks from the date received in the engineers office.

jamb architec
P.O. Box 310 Forest Lake, Minnesota 5
(612) 670-

I hereby certify that this plan, specification, or report was prepared by me or under my direct supervision and that I am a duly Registered Professional Engineer under the laws of the State of Minnesota.
Date 9/18/08 Reg. No.

1st CHURCH of CHRIST SCIENTIST

CHURCH ROOF RE-DESIGN & REPLACEMENT
ST. PAUL
MINNESOTA

Project No. 0810
Date: 9/18/08
Revisions:

Sheet No. S-1

A. Prefinished metal flashing and keepers (copper to match existing)

B. Prefinshed gutters and downspouts (copper to match existing)

C. Existing Brick

D. New Brick (to match existing)

E. New or existing trim – painted (to match existing)

F. Prefinished aluminum louver vents (w/ insect screens)

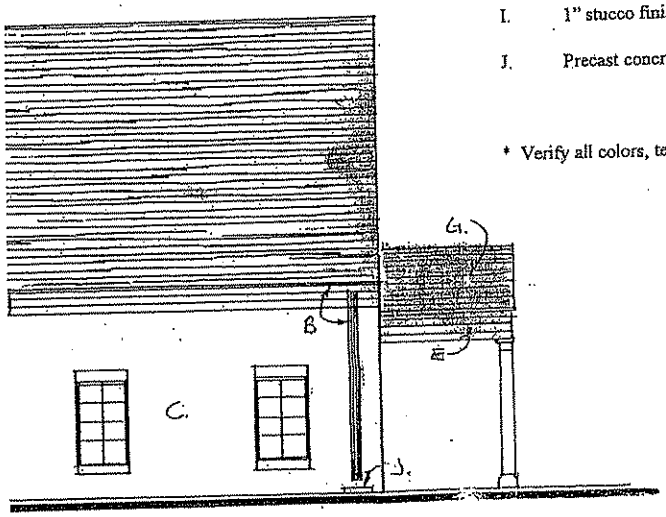
G. 340# fiberglass reinforced asphalt shingles

H. Existing stucco finish

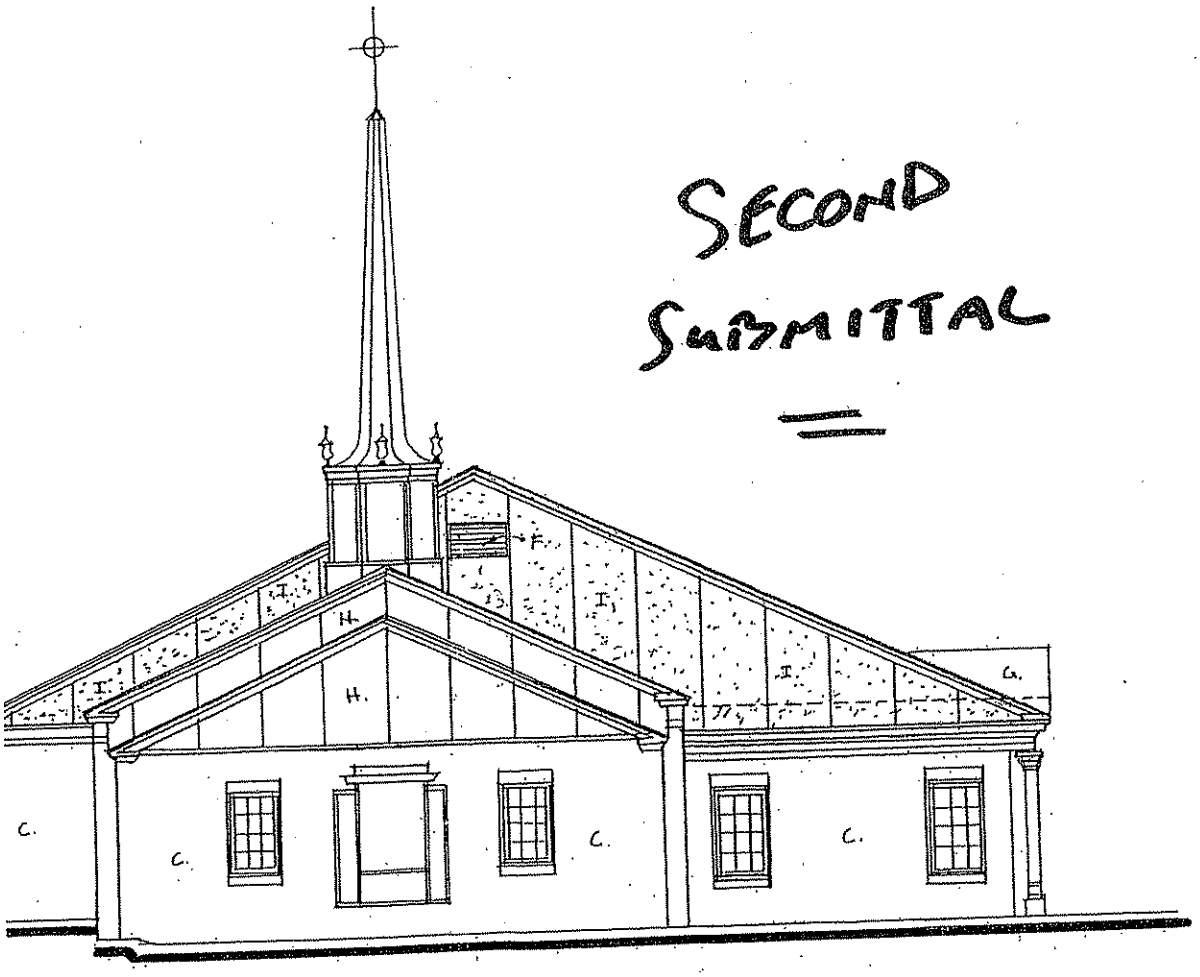
I. 1" stucco finish (texture & color to match existing stucco)

J. Precast concrete splashblocks

* Verify all colors, textures, finishes and material selections with Owner prior to application



SECOND SUBMITTAL



WEST ELEVATION
1/8" = 1'-0"

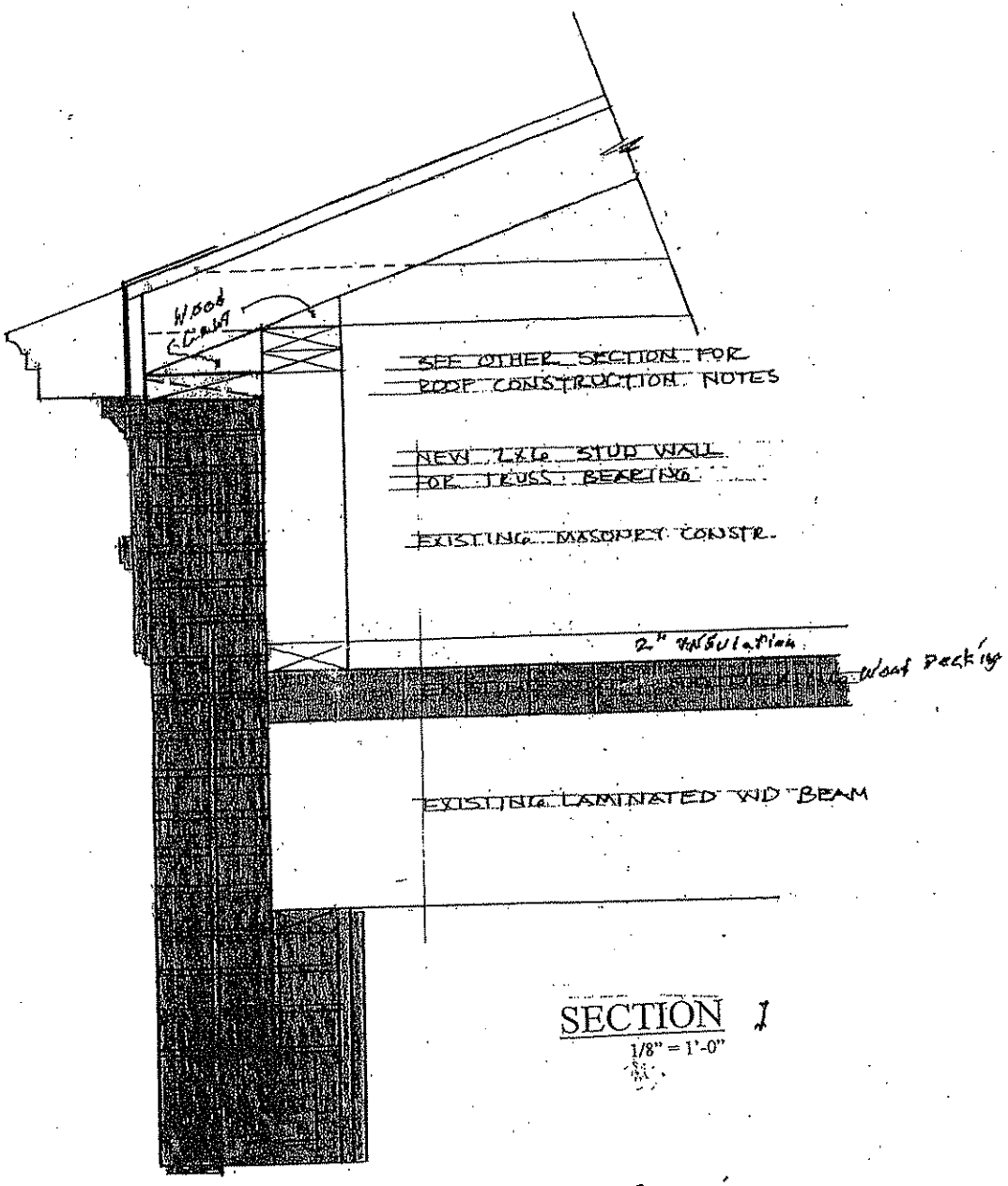
jlamb archite
P.O. Box 310 Forest Lake, Minnesot
(612) 6

I hereby certify that this plan was prepared by me or under my direct supervision and that I am a duly registered ARCHITECT under the laws of the State of MINNESOTA
Date 6/10/08 Reg. No. 12639
JEFFREY J. ANDERSON

1st CHURCH of CHRIST SCIENTIST
CHURCH ROOF RE-DESIGN & REPLACEMENT
ST. PAUL MINNESOTA

Project No. 0810
Date: 9/18/08
Revisions:

Sheet No. A-1



SECTION 1
1/8" = 1'-0"

Wall Footing

Roof - 32.7 x 55 = 1799
12" Blk. Wall - 14 x 100 = 1400
Conc. Ftg. x 150 x 5.75 = 263
3462

3462 ÷ 2000 = 1.73' OK
Ftg. Size = 1.75'

Jamb architects
P.O. Box 390 Forest Lake, MN

I hereby certify that this plan was prepared by me or under my direct supervision and that I am a duly Registered Architect under the laws of the State of Minnesota
Date 9/18/08 Reg. No. 13622

1st CHURCH of CHRIST SCIENTIST
CHURCH ROOF RE-DESIGN & REPLACEMENT

Project No. 0810
Date: 9/18/08
Revisions:

Sheet No. A-2

...rnational Building Code w/ State of Minnesota Appendix

...ations shall bear on natural undisturbed soil or on compacted granular fill.
...ndations for subcutting soils in the footing areas referred to in the soil
...ntioned above shall be strictly followed.
...he bottom of the footings as detailed is of questionable bearing value, the
...r engineers office shall be notified at once.
...ng elevation changes shall be stepped at a ratio of 1 (vert) to 2 (horiz).
...mum vertical step shall be 1'-4" unless noted otherwise.
...or wall footings shall have a minimum soil cover of 3'-8" measured from
...n of the footing unless otherwise noted.
...eport for anticipated settlement values. The structural engineer shall verify
...ettlement criteria will not be detrimental to the building or it's operation.

...s
...ds

...Areas, corridors and stairs	100 psf
...reas	*10 psf + 50 psf
...ge	125 psf
...ads	***40 psf
...ds	90 mph
...ds	Material Self Weights

...oads
...ent weight if heavier
...ng and/or sliding snow

SES:

...rength	Type Mix	Location
...psi	Std. Weight	Interior slabs and walls
...psi	Std. Weight Air Entrained	Exterior slabs and walls
...psi	Std. Weight	Footings
...psi	Std. Weight	Interior Topping
	f'm = 1,500 psi (28 days)	
...ment	Fy = 60,000 psi	ASTM A615
...l Steel	Fy = 36,000 psi	ASTM A36
...l Tubing	Fy = 46,000 psi	ASTM A500, Grd. B
...Pipe	Fy = 36,000 psi	ASTM A53, Grd. B
	Fy = 36,000 psi	ASTM A36
	Fu = 120,000 psi	ASTM A325
...bolts	Fu = 60,000 psi	ASTM A307
...ctrode	Fy = 70,000 psi	
...Wire Fabric		ASTM A185

...VERAGE FOR REINFORCEMENT:

	3"
...rete	2"
...l	Center of Slab
	Center of Wall

...VERAGE FOR REINFORCEMENT:

Center Line (unless otherwise noted)
3" to ties

...STEEL:
...rcing steel contractor shall fabricate all reinforcement and furnish all
...s, chairs, spacer bars and supports necessary to secure the reinforcement
...wn otherwise on the plans and details.
...reinforcement shall be placed according to the CRSI "Recommended
...for Placing Reinforcing Bars".
...don and tension lap splices for cast-in-place concrete shall be 36 bar
...minimum, unless noted otherwise.
...ap splices for reinforced masonry shall be 40 bar diameters minimum,
...ed otherwise.
...l reinforcing steel in footings and concrete walls shall be continuous
...ners.
...WWF should be one mesh plus two inches at splices.
...wo #5 reinforcing bars each side around openings in concrete walls and
...rs shall extend 24" beyond the corners of the openings. Also provide 2 -
...al bars at each corner of the opening.
...ng bars shall not be welded without the approval of the structural engineer.
...ding of reinforcing steel shall be performed by welders specifically certified
...rcing steel. Prior to welding the "carbon equivalent" (CE) of steel shall be
...d. Reinforcing steel whose "CE" can't be identified or whose "CE" exceeds
...all not be welded. Except for reinforcing steel conforming to ASTM A-706
...g steel shall be preheated as shown in table 1.RQA 3-77. In addition, steel
...E" between 0.66% and 0.75% shall be welded only when prior qualification
...y acceptable weldability.

MASONRY:
...ad bearing masonry units shall conform to ASTM C90 Grade N Type I, in
...o the requirements of the quality control standards of the Concrete/Masonry
...on, minimum required compressive strength of block units shall be 2800 psi
...net area)
Units shall have been cured for not less than 28 days when placed in the

...of admixtures will not be permitted in the grout or mortar unless substantiat-
...is submitted to and approved by the structural engineer or the architect.
...vertically reinforced masonry walls and bond beams shall have fc = 3,000
...days and shall have a slump of 9" to 10".
...nry walls shall have horizontal reinforcing consisting of galvanized standard
...ge. "Dur-O-Wall" or equal. All reinforcing shall be located every other course
...ded otherwise.
...s shown shall match the size and number of footing reinforcing unless noted
...1. Hook into footing 12" minimum and lap 30 diameters with main steel.
...is required for bearing walls below grade. Type "N" mortar is required for
...walls above grade. Mortar shall be of a portland cement type mix.
...itectural Plan for location and detail of vertical control joints.
...m spacing for control joints in masonry walls shall not exceed 24'-0" o.c.
...beams bearing on masonry shall have two cores minimum filled with grout
...elow the bearing point, except as noted on the plans.
...ms 8" or smaller use one #5 continuous. Bond beams 10" or larger use 2 -
...uous unless noted otherwise.
...prisms shall be performed in accordance with ASTM E447-84. Prepare 1
...isms prior to the start of construction. Test 2 of these a 7 days and the
...3 at 28 days. Prepare 1 additional set of three prisms for every 5000 sf
...ction. Test 1 of these at 7 days and the remaining 2 at 28 days. Refer to
...nspections" in the structural notes and to the project specifications for
...l information.

...RED WOOD TRUSSES:
...ber for wood trusses shall be in accordance with manufacturer's written recommendations
...russes shall have bridging and bracing in accordance with manufacturer's requirements.
...russes shall be designed for a top chord live load as noted under Design Live Loads plus
...ad/drift in accordance with applicable building codes (minimum top chord dead load shall
...sf, minimum bottom chord dead load shall be 10 psf).
...ad deflection limitations – roof trusses less than L/360.
...complete shop drawings for approval, showing the erection plan, all bearing conditions,
...nections. Calculations certified by a Professional Engineer will be required for all wood
...connections and bearing attachments/components. (Submit certified plans and calcula-
...the local building official).
...russ supplier shall detail end bearing of trusses so as not to exceed perpendicular to grain
...of wood plates that support the trusses.

...
e. All sills and plates resting on concrete or masonry, which is in contact with the earth or resting on foundations shall be pressure treated Southern Pine No. 2.
f. All bolt heads and nuts bearing on wood shall have standard cut washers. All bolt holes in wood shall be drilled 1/32" larger in diameter than nominal bolt diameters.
g. Bolts in wood shall not be less than 7 diameters from the end & 4 diameters from edge of member.
h. All framing anchors, post caps, bases, hangers, straps, etc., shall be as manufactured by "Simpson" Company or approved equal.
i. Top plates of all wood stud walls to be 2 – 2x (same width as studs), lap 48" (min.) with not less than 6-16d nails at each lap and not more than 16" between nails. Splice at studs only.
j. Moisture content of wood at time of placing shall not exceed 19%.
k. All member sizes given on the drawings are nominal sizes.
l. Spacing of bridging for joists shall not exceed 8'-0".
m. Wood lintels and headers shall have a full 3" length of bearing at each end (min.).
n. Double all joists under parallel partitions.
o. All beams and joists not bearing on supporting members shall be framed with "Simpson HHUS" joist hangers or equal. Use type as required for the application shown on the drawings.
p. Wood joists shall bear the full width of supporting members (stud walls, beams, etc.) unless otherwise indicated.
q. Wood beams made of 2 or more 2x's shall be nailed together with 3 rows of 16d nails at 12" o.c. For a 3 piece member, install the specified nailing on each side.
r. Unless noted, 4" wide stud walls are to have 2 x 4 studs at 16" o.c.
s. Unless noted, exterior walls are to be 2 x 6 studs at 16" o.c.
t. Sill plates to be bolted to foundation walls with 5/8" round bolts at 6'-0" o.c. maximum, bolts to Extend 15" minimum into grouted masonry. Each sill plate to have a minimum of 2 bolts with one bolt located within 12" of each end of each plate.

11. NAILING SCHEDULE:
a. joists or rafters to sides of studs
8" joist or less 3 -16d
for each additional 4" in depth of joist 1 – 16d
b. bridging to joists, toenail each end 2 – 8d
blocking between joists or rafters to joists or rafters – toenail each side, each end 2 – 10d
blocking between studs each end 2 – 10d toenails or 2 – 16d
c. 1" x 6" subfloor to each joist, face nail 2 – 8d
d. wider than 1" x 6" to each joist, face nail 3 – 8d
e. 2" subfloor to joist or girder, blind and face nail 2 – 16d
f. Sole plate to joist or blocking, face nail 16d @ 16" o.c.
g. Top plate to stud, end nail 2 – 16d
h. Stud to sole plate 4 – 10d toenail, or 2 – 16d end nail
i. Double top plates, face nail 16d @ 16" o.c.
j. Continuous header, two pieces 16d @ 12" o.c. along each edge
k. Ceiling joists to plate, toenail 3 – 8d
l. Continuous header to studs, toenail 4 – 8d
m. Ceiling joists, laps over partitions, face nail 3 – 16d
n. Ceiling joists to parallel rafters, face nail 3 - 16d
o. Joists or rafters at all bearing, toenail each side 2 -10d
p. 1" brace to each stud and plate, face nail 2- 8d
q. 1" x 8" sheathing or less to each bearing, face nail 2 – 8d
r. wider than 1" x 8" sheathing to each bearing, face nail 3- 8d
s. Built-up corner studs 16d @ 24" o.c.
t. Double studs 10d @ 12" o.c. direct
u. Rim joists 6" or less 16d (2/ joist) end nail
v. Rim joists 6" or more 16d (3/ joist) end nail
w. Diagonal bracing (to stud & plate) 16d (3/ joist) end nail

12. BACKFILLING:
a. No backfilling and compacting of earth shall be permitted against foundation walls until supporting slabs have been poured and have reached 75% of their design strength or unless adequate bracing submitted for review has been approved.
b. Both sides of foundation walls shall be backfilled simultaneously so as to prevent overturning or lateral movement of walls.
c. All grade beams shall be adequately braced to prevent lateral movement during backfilling and compaction.

13. CONSTRUCTION AND CONTROL JOINTS:
a. Construction joints shall be made as detailed on the drawings.
b. Maximum spacing for control joints shall be 15'-0".
c. A 15'-0" maximum spacing of control joints may not insure complete control of shrinkage cracks. A closer spacing may be used by request of the contractor if more complete shrinkage crack control is desired. Contractor to verify with Owner.

14. EXPANSION BOLTS:
a. All expansion bolts shall be KWIK bolts or REDHEAD as noted on the plans. Minimum embedment unless otherwise noted shall be 4" for 1/2" diameter bolts, and 5" for 5/8" and 3/4" diameter bolts.

15. CONSTRUCTION PROCEDURES:
a. The structure shall be adequately braced and shored against wind and construction loads during erection. Structural members are designed for "in-place" loads only.
b. Comply with all applicable city, state and federal laws, including Occupational Safety and Health Administration Act (OSHA) and regulations adopted pursuant thereto.
c. The contract structural drawings and specifications represent the finished structure. Unless otherwise noted, they do not indicate the means or method of construction. Provide all measures necessary to protect the structure, workmen, or other persons during construction. Such measures shall include, but are not limited to, bracing, shoring for construction equipment, shoring for earth loads, forms, scaffolding, planking, safety nets, support and bracing for cranes and gin poles, etc.
d. Engage properly qualified persons to determine where and how temporary precautionary measures shall be used and inspect same in the field. Observation visits to the jobsite by the ENGINEER'S representative shall not induce inspection of the above items.
e. Supervise and direct the work so as to maintain sole responsibility for all construction means, methods, techniques, sequences, and procedures. As a part of this responsibility, retain the services of a licensed structural engineer to design and supervise any scaffolding for workmen, and all shoring of forms and elements of the construction.

16. COORDINATION WITH ARCHITECTURAL DRAWINGS:
a. The contractors shall verify all dimensions and elevations with the architectural drawings. Where discrepancies occur, it is the contractors responsibility to notify the Architect prior to proceeding with construction.

17. NEW WORK IN CONJUNCTION WITH EXISTING CONSTRUCTION:
a. The contractors shall verify, by field check, all sizes, dimensions, elevations, locations, etc. of elements of the existing construction which are relative to the new construction.
b. All dimensions involving new work tying into or governed by existing construction shall be field checked by the contractors and furnished to the subcontractors prior to fabrication of any work. The verified dimensions shall appear and be noted as such on the first shop drawings submitted.
c. Cutting of existing structural steel is prohibited with out approval of the engineer.

18. SHOP DRAWINGS:
a. Shop drawings, unless otherwise noted, shall be submitted on reproducible transparencies with one print for review prior to fabrication. Send reproducible transparencies and print in a mailing tube. Transparencies which will not produce a legible print shall be returned for resubmittal.
b. Shop drawings are to be prepared under the supervision of a registered professional engineer. They are to include complete details and schedules for fabrication and assembly of structural members, procedures and diagrams.
c. Fabricators shall draw their own erection plans. Copying the structural plans and using them as erection plans is not acceptable.
d. Prior to submittal, the contractors shall review the shop drawings and make any corrections required. The contractors shall stamp and sign the drawings as evidence that they have reviewed them.
e. Shop drawings shall be submitted for all structural components.
f. Turn around time for shop drawings shall be two weeks from the date received in the engineers office.

jlamb archite
P.O. Box 310 Forest Lake, Minnesota (612) 67

I hereby certify that this plan, specification, or report was prepared by me or under my direct supervision and that I am a duly Registered Professional Engineer under the laws of the State of Minnesota.

Date 9/18/08 Rev. No.

1st CHURCH of CHRIST SCIENTIST
CHURCH ROOF RE-DESIGN & REPLACEMENT
ST. PAUL MINNESOTA

Project No. 0810
Date: 9/18/08
Revisions:

Sheet No. S-1

A. Prefinished metal flashing and keepers (copper to match existing)

B. Prefinshed gutters and downspouts (copper to match existing)

C. Existing Brick

D. New Brick (to match existing)

E. New or existing trim – painted (to match existing)

F. Prefinished aluminum louver vents (w/ insect screens)

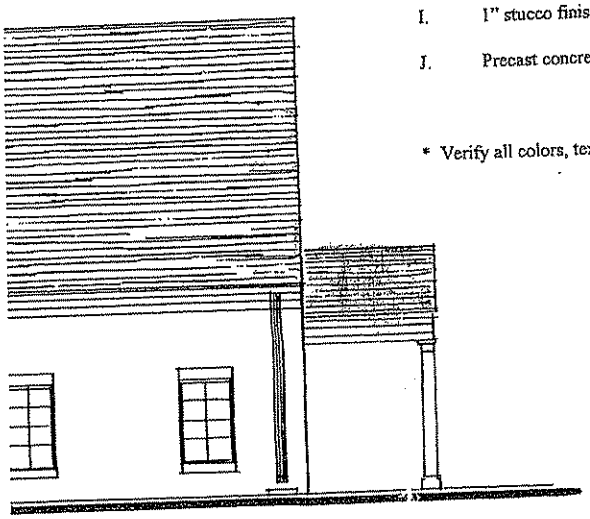
G. 340# fiberglass reinforced asphalt shingles

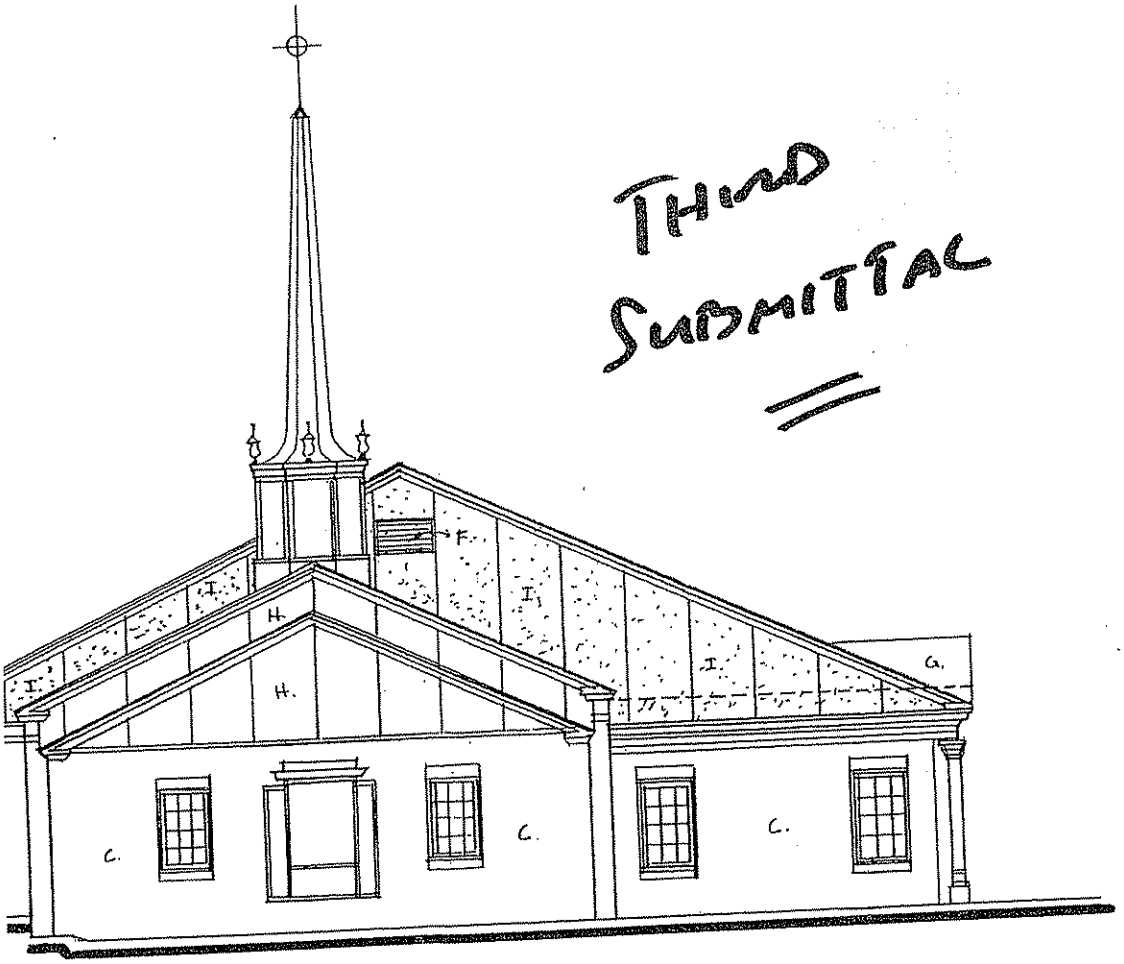
H. Existing stucco finish

I. 1" stucco finish (texture & color to match existing stucco)

J. Precast concrete splashblocks

* Verify all colors, textures, finishes and material selections with Owner prior to application





WEST ELEVATION
1/8" = 1'-0"

Jamb architec
P.O. Box 310 Forest Lake, Minnesota (612) 67C

I hereby certify that this plan was prepared by me or under my direct supervision and that I am a duly registered Architect under the laws of the State of Minnesota

Date 8/18/08 Reg. No. 12639

JEFFREY J. ANDERSON

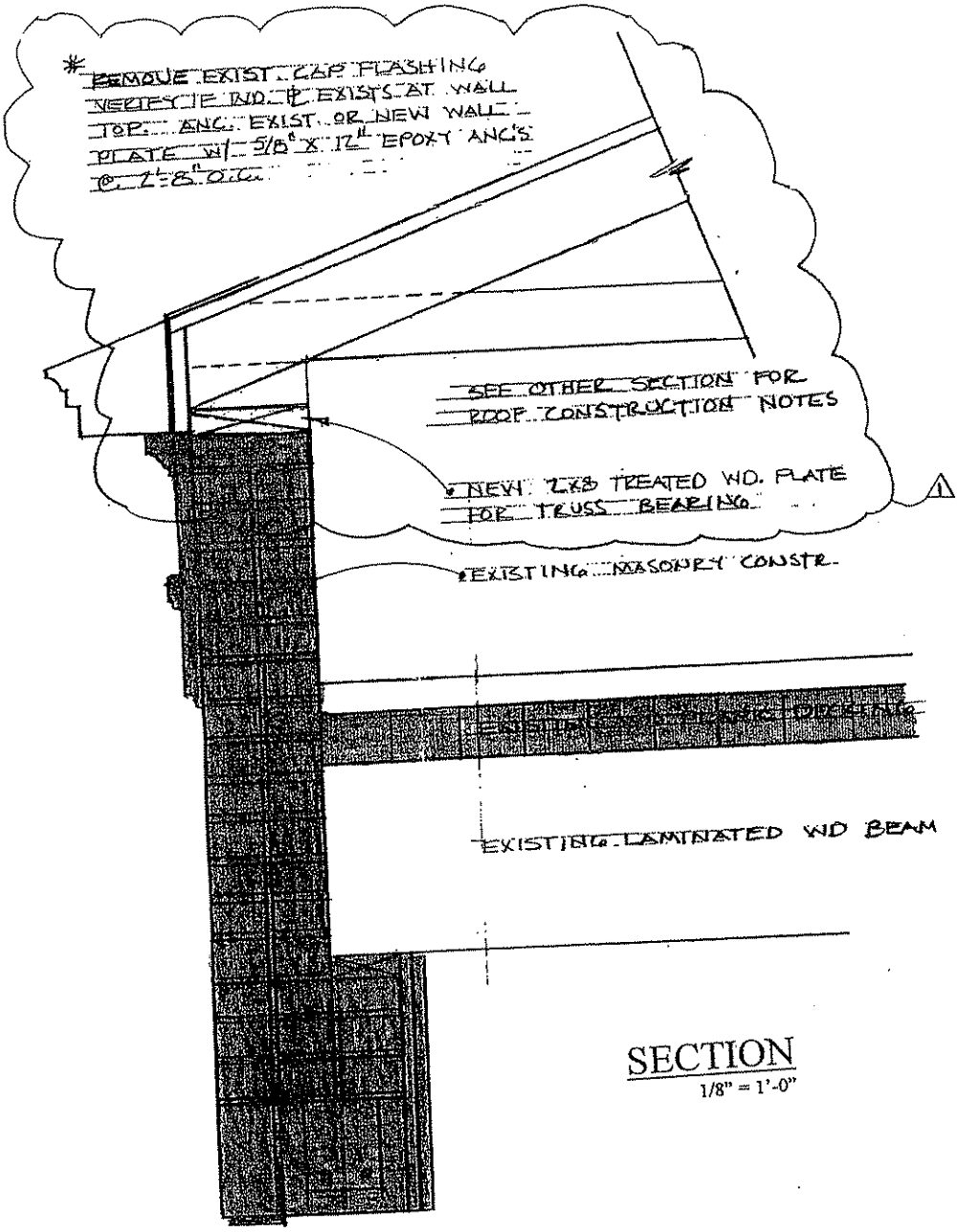
1st CHURCH of CHRIST SCIENTIST
CHURCH ROOF RE-DESIGN & REPLACEMENT
MINNESOTA
ST. PAUL

Project No. 0810

Date: 9/18/08

Revisions:

Sheet No. A-1



SECTION
1/8" = 1'-0"

Jamb architects
P.O. Box 310 Forest Lake, Minn

I hereby certify that this plan was prepared by me or under my direct supervision and that I am a duly registered ARCHITECT under the laws of the State of Minnesota.
Date 9/18/08 Reg. No. 13689
Jerry W. Anderson

1st CHURCH of CHRIST SCIENTIST
CHURCH ROOF RE-DESIGN & REPLACEMENT
MINNESOTA
ST PAUL

Project No. 0810
Date: 9/18/08
Revisions:
1 BEARING 10/3/08
Sheet No. A-2

shall bear on natural undisturbed soil or on compacted granular fill.
ns for subcutting soils in the footing areas referred to in the soil
l above shall be strictly followed.
ion of the footings as detailed is of questionable bearing value, the
neers office shall be notified at once.
ation changes shall be stepped at a ratio of 1 (vert) to 2 (horiz).
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footings shall have a minimum soil cover of 3'-8" measured from
a footing unless otherwise noted.
or anticipated settlement values. The structural engineer shall verify
nt criteria will not be detrimental to the building or it's operation.

, corridors and stairs	100 psf
	*10 psf + 50 psf
	125 psf
	***40 psf
	90 mph
	Material Self Weights

ight if heavier
or sliding snow

Type Mix	Location
Std. Weight	Interior slabs and walls
Std. Weight	Exterior slabs and walls
Air Entrained	
Std. Weight	Footings
Std. Weight	Interior Topping
f'm = 1,500 psi (28 days)	
Fy = 60,000 psi	ASTM A615
Fy = 36,000 psi	ASTM A36
ng Fy = 46,000 psi	ASTM A500, Grd. B
Fy = 36,000 psi	ASTM A53, Grd. B
Fy = 36,000 psi	ASTM A36
Fu = 120,000 psi	ASTM A325
Fu = 60,000 psi	ASTM A307
Fy = 70,000 psi	
abric	ASTM A185

\OE FOR REINFORCEMENT:
3"
2"
Center of Slab
Center of Wall

\GE FOR REINFORCEMENT:
Center Line (unless otherwise noted)
3" to ties

L:
steel contractor shall fabricate all reinforcement and furnish all
rs, spacer bars and supports necessary to secure the reinforcement
terwise on the plans and details.
cement shall be placed according to the CRSI "Recommended
cing Reinforcing Bars".
d tension lap splices for cast-in-place concrete shall be 36 bar
mum, unless noted otherwise.
ces for reinforced masonry shall be 40 bar diameters minimum,
erwise.
orcing steel in footings and concrete walls shall be continuous

F should be one mesh plus two inches at splices.
reinforcing bars each side around openings in concrete walls and
l extend 24" beyond the corners of the openings. Also provide 2 -
s at each corner of the opening.
s shall not be welded without the approval of the structural engineer.
f reinforcing steel shall be performed by welders specifically certified
steel. Prior to welding the "carbon equivalent" (CE) of steel shall be
inforcing steel whose "CE" can't be identified or whose "CE" exceeds
be welded. Except for reinforcing steel conforming to ASTM A-706
shall be preheated as shown in table 1.RCSA 3-77. In addition, steel
ween 0.66% and 0.75% shall be welded only when prior qualification
ptable weldability.

ONRY:
uring masonry units shall conform to ASTM C90 Grade N Type I, in
equirements of the quality control standards of the Concrete/Masonry
imum required compressive strength of block units shall be 2800 psi
ea)
shall have been cured for not less than 28 days when placed in the

ixtures will not be permitted in the grout or mortar unless substantial-
mitted to and approved by the structural engineer or the architect.
ally reinforced masonry walls and bond beams shall have fc = 3,000
and shall have a slump of 9" to 10".
ills shall have horizontal reinforcing consisting of galvanized standard
bar-O-Wall" or equal. All reinforcing shall be located every other course
nerwise.
on shall match the size and number of footing reinforcing unless noted
k into footing 12" minimum and lap 30 diameters with main steel.
uired for bearing walls below grade. Type "N" mortar is required for
bove grade. Mortar shall be of a portland cement type mix.
al Plan for location and detail of vertical control joints.
ing for control joints in masonry walls shall not exceed 24'-0" o.c.
bearing on masonry shall have two cores minimum filled with grout
he bearing point, except as noted on the plans.
or smaller use one #5 continuous. Bond beams 10" or larger use 2 -
unless noted otherwise.
s shall be performed in accordance with ASTM E447-84. Prepare 1
rior to the start of construction. Test 2 of these a 7 days and the
28 days. Prepare 1 additional set of three prisms for every 5000 sf
Test 1 of those at 7 days and the remaining 2 at 28 days. Refer to
tions" in the structural notes and to the project specifications for
mation.

WOOD TRUSSES: @ 15,000
r wood trusses shall be in accordance with manufacturer's written recommendations
shall have bridging and bracing in accordance with manufacturer's requirements.
shall be designed for a top chord live load as noted under Design Live Loads plus
ft in accordance with applicable building codes (minimum top chord dead load shall
imum bottom chord dead load shall be 10 psf.).
lection limitations – roof trusses less than L/360.
mit shop drawings for approval, showing the erection plan, all bearing conditions,
ers. Calculations certified by a Professional Engineer will be required for all wood
ections and bearing attachments/components. (Submit certified plans and calcula-
cal building official).
upplier shall detail end bearing of trusses so as not to exceed perpendicular to grain
od plates that support the trusses.

on foundations shall be pressure treated Southern Pine No. 2.
f. All bolt heads and nuts bearing on wood shall have standard cut washers. All bolt holes in wood shall be drilled 1/32" larger in diameter than nominal bolt diameters.
g. Bolts in wood shall not be less than 7 diameters from the end & 4 diameters from edge of member.
h. All framing anchors, post caps, bases, hangers, straps, etc., shall be as manufactured by "Simpson" Company or approved equal.
i. Top plates of all wood stud walls to be 2 – 2x (same width as studs), lap 48" (min.) with not less than 6-16d nails at each lap and not more than 16" between nails. Splice at studs only.
j. Moisture content of wood at time of placing shall not exceed 19%.
k. All member sizes given on the drawings are nominal sizes.
l. Spacing of bridging for joists shall not exceed 8'-0".
m. Wood lintels and headers shall have a full 3" length of bearing at each end (min.).
n. Double all joists under parallel partitions.
o. All beams and joists not bearing on supporting members shall be framed with "Simpson HHUS" joist hangers or equal. Use type as required for the application shown on the drawings.
p. Wood joists shall bear the full width of supporting members (stud walls, beams, etc.) unless otherwise indicated.
q. Wood beams made of 2 or more 2x's shall be nailed together with 3 rows of 16d nails at 12" o.c. For a 3 piece member, install the specified nailing on each side.
r. Unless noted, 4" wide stud walls are to have 2 x 4 studs at 16" o.c.
s. Unless noted, exterior walls are to be 2 x 6 studs at 16" o.c.
t. Sill plates to be bolted to foundation walls with 5/8" round bolts at 6'-0" o.c. maximum, bolts to Extend 15" minimum into grouted masonry. Each sill plate to have a minimum of 2 bolts with one bolt located within 12" of each end of each plate.

11. NAILING SCHEDULE:
a. joists or rafters to sides of studs
8" joist or less 3 -16d
For each additional 4" in depth of joist 1 – 16d
b. bridging to joists, toenail each end 2 – 8d
blocking between joists or rafters to joists or rafters – toenail each side, each end 2 – 10d
blocking between studs each end 2 – 10d toenails or 2 – 16d
c. 1" x 6" subfloor to each joist, face nail 2 – 8d
d. wider than 1" x 6" to each joist, face nail 3 – 8d
e. 2" subfloor to joist or girder, blind and face nail 2 – 16d
f. Sole plate to joist or blocking, face nail 16d @ 16" o.c.
g. Top plate to stud, end nail 2 – 16d
h. Stud to sole plate 4 – 10d toenail, or 2 – 16d end nail
i. Double top plates, face nail 16d @ 16" o.c.
j. Continuous header, two pieces 16d @ 12" o.c. along each edge
k. Ceiling joists to plate, toenail 3 – 8d
l. Continuous header to studs, toenail 4 – 8d
m. Ceiling joists, laps over partitions, face nail 3 – 16d
n. Ceiling joists to parallel rafters, face nail 3 - 16d
o. Joists or rafters at all bearing, toenail each side 2 -10d
p. 1" brace to each stud and plate, face nail 2- 8d
q. 1" x 8" sheathing or less to each bearing, face nail 2 – 8d
r. wider than 1" x 8" sheathing to each bearing, face nail 3- 8d
s. Built-up corner studs 16d @ 24" o.c.
t. Double studs 10d @ 12" o.c. direct
u. Rim joists 6" or less 16d (2/ joist) end nail
v. Rim joists 6" or more 16d (3/ joist) end nail
w. Diagonal bracing (to stud & plate) 16d (3/ joist) end nail

12. BACKFILLING:
a. No backfilling and compacting of earth shall be permitted against foundation walls until supporting slabs have been poured and have reached 75% of their design strength or unless adequate bracing submitted for review has been approved.
b. Both sides of foundation walls shall be backfilled simultaneously so as to prevent overturning or lateral movement of walls.
c. All grade beams shall be adequately braced to prevent lateral movement during backfilling and compaction.

13. CONSTRUCTION AND CONTROL JOINTS:
a. Construction joints shall be made as detailed on the drawings.
b. Maximum spacing for control joints shall be 15'-0".
c. A 15'-0" maximum spacing of control joints may not insure complete control of shrinkage cracks. A closer spacing may be used by request of the contractor if more complete shrinkage crack control is desired. Contractor to verify with Owner.

14. EXPANSION BOLTS:
a. All expansion bolts shall be KWIK bolts or REDHEAD as noted on the plans. Minimum embedment unless otherwise noted shall be 4" for 1/2" diameter bolts, and 5" for 5/8" and 3/4" diameter bolts.

15. CONSTRUCTION PROCEDURES:
a. The structure shall be adequately braced and shored against wind and construction loads during erection. Structural members are designed for "in-place" loads only.
b. Comply with all applicable city, state and federal laws, including Occupational Safety and Health Administration Act (OSHA) and regulations adopted pursuant thereto.
c. The contract structural drawings and specifications represent the finished structure. Unless otherwise noted, they do not indicate the means or method of construction. Provide all measures necessary to protect the structure, workmen, or other persons during construction. Such measures shall include, but are not limited to, bracing, shoring for construction equipment, shoring for earth loads, forms, scaffolding, planking, safety nets, support and bracing for cranes and gin poles, etc.
d. Engage properly qualified persons to determine where and how temporary precautionary measures shall be used and inspect same in the field. Observation visits to the jobsite by the ENGINEER'S representative shall not include inspection of the above items.
e. Supervise and direct the work so as to maintain sole responsibility for all construction means, methods, techniques, sequences, and procedures. As a part of this responsibility, retain the services of a licensed structural engineer to design and supervise any scaffolding for workmen, and all shoring of forms and elements of the construction.

16. COORDINATION WITH ARCHITECTURAL DRAWINGS:
a. The contractors shall verify all dimensions and elevations with the architectural drawings. Where discrepancies occur, it is the contractors responsibility to notify the Architect prior to proceeding with construction.

17. NEW WORK IN CONJUNCTION WITH EXISTING CONSTRUCTION:
a. The contractors shall verify, by field check, all sizes, dimensions, elevations, locations, etc. of elements of the existing construction which are relative to the new construction.
b. All dimensions involving new work tying into or governed by existing construction shall be field checked by the contractors and furnished to the subcontractors prior to fabrication of any work. The verified dimensions shall appear and be noted as such on the first shop drawings submitted.
c. Cutting of existing structural steel is prohibited with out approval of the engineer.

18. SHOP DRAWINGS:
a. Shop drawings, unless otherwise noted, shall be submitted on reproducible transparencies with one print for review prior to fabrication. Send reproducible transparencies and print in a mailing tube. Transparencies which will not produce a legible print shall be returned for resubmittal.
b. Shop drawings are to be prepared under the supervision of a registered professional engineer. They are to include complete details and schedules for fabrication and assembly of structural members, procedures and diagrams.
c. Fabricators shall draw their own erection plans. Copying the structural plans and using them as erection plans is not acceptable.
d. Prior to submittal, the contractors shall review the shop drawings and make any corrections required. The contractors shall stamp and sign the drawings as evidence that they have reviewed them.
e. Shop drawings shall be submitted for all structural components.
f. Turn around time for shop drawings shall be two weeks from the date received in the engineers office.

jamb architects
P.O. Box 310 Forest Lake, Minnesota (612)

I hereby certify that this plan, specification, or report was prepared by me or under my direct supervision and that I am a duly Registered Professional Engineer under the laws of the State of Minnesota.
Date: 9/18/08 Reg. No.

1st CHURCH of CHRIST SCIENTIST
CHURCH ROOF RE-DESIGN & REPLACEMENT
ST. PAUL MINNESOTA

Project No. 0810
Date: 9/18/08
Revisions:

Sheet No. S-1

DEPARTMENT OF SAFETY AND INSPECTIONS
Bob Kessler, Director

CITY OF SAINT PAUL
Christopher B. Coleman, Mayor

COMMERCE BUILDING
8 Fourth St. E, Suite 200
Saint Paul, Minnesota 55101-102

Telephone: 651-266-9090
Facsimile: 651-266-9124
Web: www.stpaul.gov/dsi

October 17, 2008

Lynette Dufresne
Minnesota Board of Architecture, Engineering, Land Surveying,
Landscape Architecture, Geoscience & Interior Design
85 E 7th Place, Suite 160
St. Paul, MN 55101-2113

Re: [redacted]
Jerry Anderson, Architect #13639

Dear Ms. Dufresne:

I am writing in my capacity as the staff structural engineer for the City of Saint Paul's Department of Safety and Inspections (DSI). This letter is regarding both [redacted] and Mr. Anderson because what I have to say all has to do with one project that was certified by both individuals.

The project consists of adding a fairly steep, pitched roof structure over what is presently a flat roof to the 1st Church of Christ Scientist.

The drawings were brought to DSI September 19, 2008, by Mr. Tim Tacheny representing the Church ownership and in pursuit of a building permit. The drawings consisted of architectural sheets A-1 and A-2 and structural sheet S-1. The architectural sheets had been certified by Mr. Anderson the day before (September 18th) and the structural sheet by [redacted] also the day before. I began my structural plan review right in my office and in the presence of Mr. Tacheny.

The drawings indicated the width of the existing church building to be 65 feet with the proposed new roof trusses spanning in that direction. Mr. Tacheny indicated to me that the existing flat roof structure spanned in the same direction utilizing a center bearing wall.

My concerns were 1) lack of detail indicating the configuration of the new trusses, specifically, was it the designer's intent that the new trusses bear at the center bearing wall or that they span the entire 65 feet thus doubling the load at the exterior walls, 2) load path to the foundation for either scenario, and 3) anchorage, or hold-down, with respect to wind load.

In the presence of Mr. Tacheny I called [redacted] to discuss what I would consider questions very basic to the structure of the building. With [redacted] permission I put him on speaker phone. [redacted] did not recall having certified the drawings even though his certification was dated the previous day, nor did he recall the project at all. In fact, he began discussing a totally different project.

EXHIBIT B

Mr. Tacheny took it on his own to bring the drawings to [redacted] office. [redacted] called me later that day with Mr. Tacheny at his office. It was at that time that I took up my questions with [redacted].

[redacted] did not appear familiar with the two details pertaining to load path that were indicated on architectural sheet A2. When I brought up the question of whether or not the intent was for the new trusses to bear on an existing center bearing wall his answer was "that's a good question". Hold-downs did not appear to have been considered. I left it with [redacted] to modify his drawings for re-submittal.

Mr. Tacheny returned to DSI at a later date with supposedly revised drawings. All that had been added was a hand written note pertaining to a footing size with no indication as to its significance, nor was there any reference as to where it applied. None of my questions had been addressed.

I suggested to Mr. Tacheny that we pursue answers to my structural questions through the architect. Mr. Anderson had not been very readily available until now because he was on a trip to London. It was at this point that I decided to check with the State Board on the status of Mr. Anderson's registration.

I learned that Mr. Anderson's registration had expired as of June 30, 2008. I explained to Mr. Tacheny that this needed to be cleared up by Mr. Anderson, with the Board, before I would be able to finalize my review. My meeting with Mr. Tacheny ended with a message to Mr. Anderson to call me back.

A couple days later Mr. Anderson returned my call. He understood my questions, appeared to welcome the input, and agreed to take up my questions with [redacted]. I informed Mr. Anderson of his certification having expired. Mr. Tacheny had already made him aware.

At still a later date Mr. Tacheny returned again with revised drawings. However, there were no revisions to the structural drawings. The architectural details on the A-2 sheet were revised. With Mr. Tacheny stating that he understood that the trusses would be supported at a center bearing wall and the drawings still not indicating a center wall I decided to put Mr. Anderson on a conference call.

Mr. Anderson was very profession and very responsive. With Mr. Tacheny part of the conference call a work plan was agreed to that I indicated would move us in the direction of my sign-off. The work plan involved during-construction site time for Mr. Anderson along with approval of roof truss shop drawings by Mr. Anderson, all with Mr. Anderson coordinating necessary structural input with [redacted]. It was understood by both Mr. Anderson and Mr. Tacheny, however, that there would be no more progress toward a building permit until Mr. Anderson cleared up the issue of his expired registration.

It surprised me that this later issue of the architectural drawings still had Mr. Anderson's September 18, 2008, certification.

As of the date of this letter nothing further has transpired.

Sincerely,

Frank Berg, P.E.
DSI Staff Structural Engineer

Jamb architects
Planning • Design • Construction Management

22 January, 2009

JAN 2 9 2009

Minnesota Board of AELSLAG&ID
85 East 7th Place, suite 160
St. Paul, Minnesota 55101

To Ms. Lynette DuFresne:

During the course of finalizing plans for the Church of Christ Scientist, it was brought to my attention that my Minnesota Registration as an Architect had lapsed. I received this information while out of the Country and visiting family in London. Upon returning to the U.S. I took the necessary steps to reinstate the License and paid the late fee associated with the late date of renewal.

I had moved my office in November of 2007 and found out that the postal services standard for forwarding mail is now six months instead of one year. It can be renewed again within the one year time frame. I did not receive a renewal notification and overlooked the fact that my license was up for renewal. This was definitely my fault for not notifying the Board of change of address for my business at the time of the move, but was entirely a simple oversight.

As per your request, I am submitting for your review:

1) copies of certified plans, correspondence, invoices and a varied number of proposals to the Church of Christ Scientist

2) Copies of my letterhead; I do not advertise and to my knowledge other than the Church of Christ Scientist did not send out materials during this time.

3) Copy of my business card for your review.

4) I have neither written nor published any professional articles during that time.

5) The only other project that was current during that time was the Pinnacle Building in Minneapolis. This project started construction in January of 2008 and was under construction during this time. I performed Construction Contract Administration. (Owner Nic Thomley 612.730.3592).

If you should require additional information or would like to discuss any of the enclosed information with me, please feel free to contact me at 612.670.0358.

Sincerely,

Jerry W. Anderson
Jamb Architects, Inc.

EXHIBIT C

P.O. Box 310 • Forest Lake, MN 55025 • Cell: 612-670-0358

A. Prefinished metal flashing and keepers (copper to match existing)

B. Prefinshed gutters and downspouts (copper to match existing)

C. Existing Brick

D. New Brick (to match existing)

E. New or existing trim – painted (to match existing)

F. Prefinished aluminum louver vents (w/ insect screens)

G. 340# fiberglass reinforced asphalt shingles

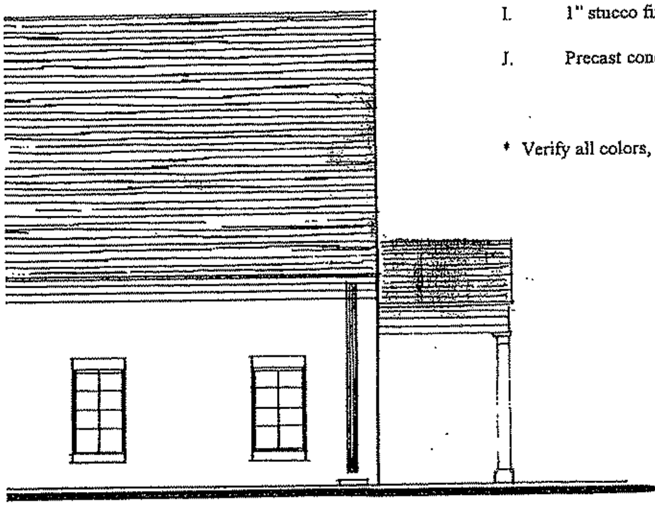
H. Existing stucco finish

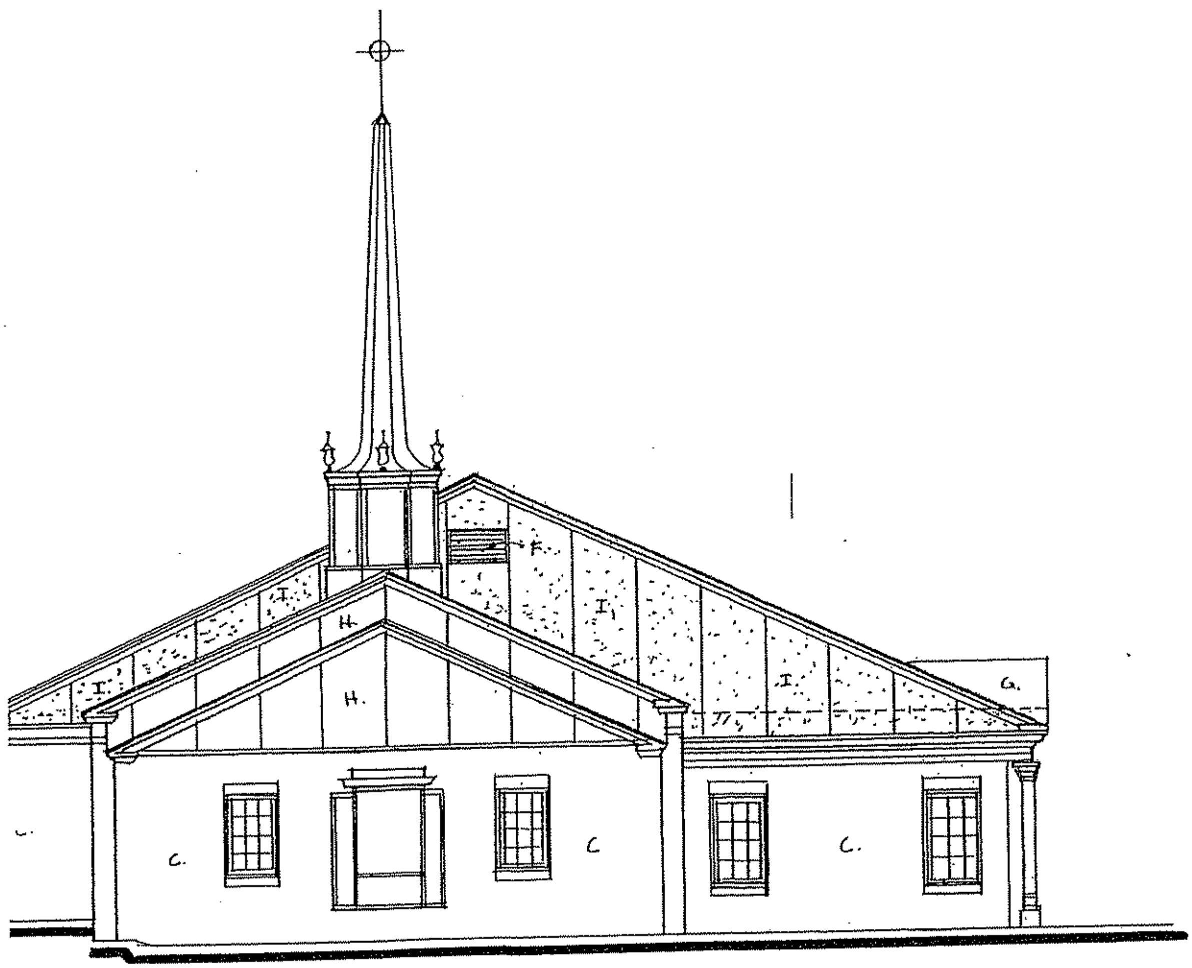
I. 1" stucco finish (texture & color to match existing stucco)

J. Precast concrete splashblocks

JAN 2 9 2009

* Verify all colors, textures, finishes and material selections with Owner prior to application





WEST ELEVATION

1/8" = 1'-0"

Jamb architect
P.O. Box 310 Forest Lake, Minnesota 55
(612) 670-0

I hereby certify that this plan was prepared by me or under my direct supervision and that I am a duly Registered ARCHITECT under the laws of the State of MINNESOTA

Date: Reg. No. 13639

JERRY J. ANDERSON

1st CHURCH of CHRIST SCIENTIST

CHURCH ROOF RE-DESIGN & REPLACEMENT
ST. PAUL MINNESOTA

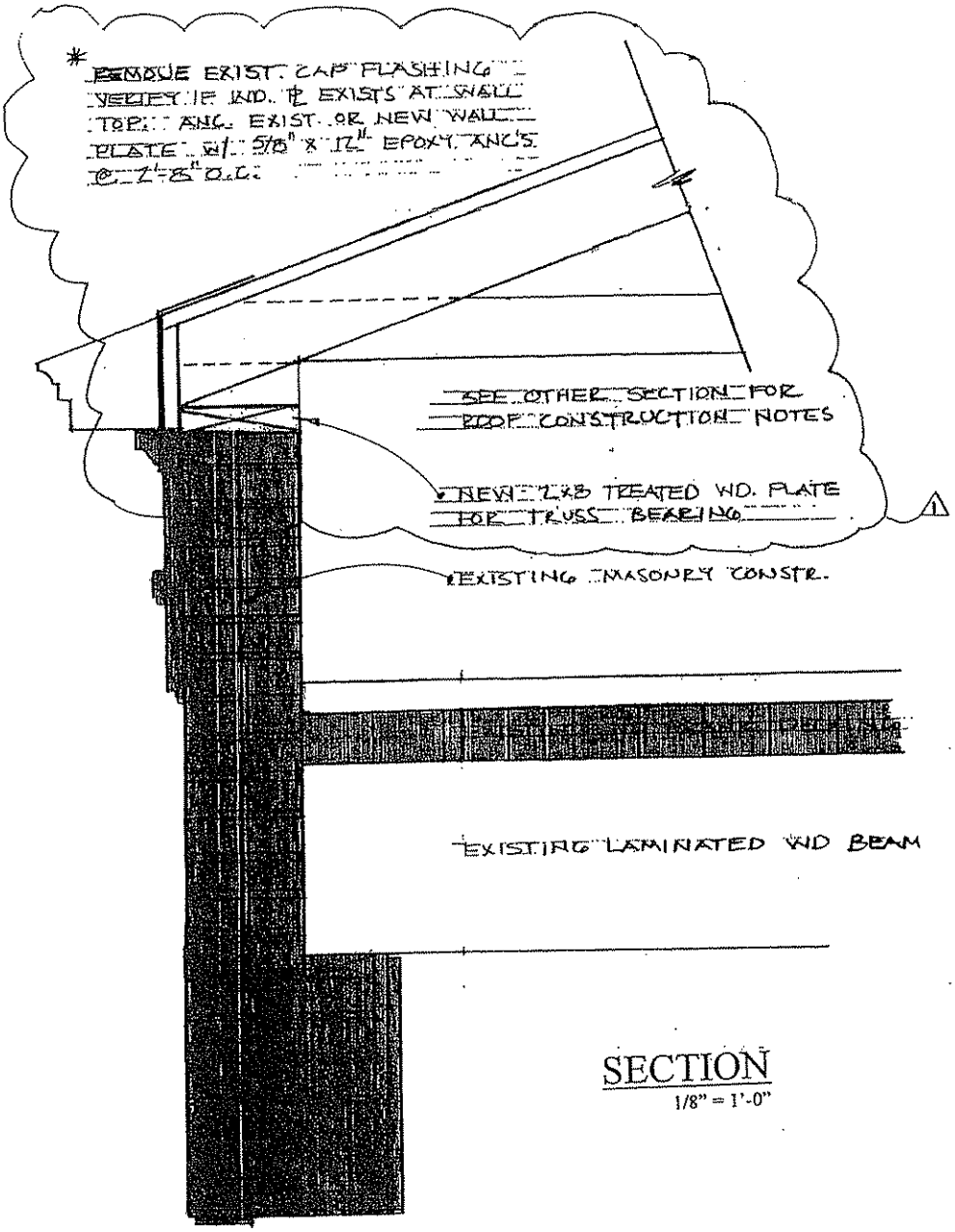
Project No. 0810

Date: 9/18/08

Revisions:

Sheet No. A-1

JAN 2 9 2009



SECTION
1/8" = 1'-0"

Jamb architex
P.O. Box 310 Forest Lake, Minnesota (612) 690

I hereby certify that this plan was prepared by me or under my direct supervision and that I am a duly registered ARCHITECT under the laws of the State of MINNESOTA
Date 1/28 Reg. No. 12689
Jerry W. Anderson

1st CHURCH of CHRIST SCIENTIST

CHURCH ROOF RE-DESIGN & REPLACEMENT
ST. PAUL MINNESOTA

Project No. 0810
Date: 9/18/08
Revisions:
1 BEARING 10/3/08
Sheet No. A-2

JAN 2 9 2008

NG CODES USED FOR DESIGN:
007 International Building Code w/ State of Minnesota Appendix

ATION:
ll foundations shall bear on natural undisturbed soil or on compacted granular fill.
ecommendations for subcutting soils in the footing areas referred to in the soil
eport mentioned above shall be strictly followed.
'soil at the bottom of the footings as detailed is of questionable bearing value, the
chitect or engineers office shall be notified at once.
all footing elevation changes shall be stepped at a ratio of 1 (vert) to 2 (horiz).
e maximum vertical step shall be 1'-4" unless noted otherwise.
ll exterior wall footings shall have a minimum soil cover of 3'-8" measured from
e bottom of the footing unless otherwise noted.
e soils report for anticipated settlement values. The structural engineer shall verify
at this settlement criteria will not be detrimental to the building or it's operation.

LOADS:
ve Loads

Public Areas, corridors and stairs	100 psf
Office areas	*10 psf + 50 psf
Storage	125 psf
ow Loads	***40 psf
ind Loads	90 mph
ead Loads	Material Self Weights

nition Loads
equipment weight if heavier
e drifting and/or sliding snow

STRESSES:
ncrete

day Strength	Type Mix	Location
3500 psi	Std. Weight	Interior slabs and walls
4000 psi	Std. Weight Air Entrained	Exterior slabs and walls
3000 psi	Std. Weight	Footings
3000 psi	Std. Weight	Interior Topping

asonry fm = 1,500 psi (28 days)

einforcement	Fy = 60,000 psi	ASTM A615
uctural Steel	Fy = 36,000 psi	ASTM A36
uctural Tubing	Fy = 46,000 psi	ASTM A500, Grd. B
l. Steel Pipe	Fy = 36,000 psi	ASTM A53, Grd. B
des	Fy = 36,000 psi	ASTM A36
lts	Fu = 120,000 psi	ASTM A325
chor Bolts	Fu = 60,000 psi	ASTM A307
ld Electrode	Fy = 70,000 psi	
ded Wire Fabric		ASTM A185

TE COVERAGE FOR REINFORCEMENT:

s	3"
r Concrete	2"
Grade	Center of Slab
walls	Center of Wall

Y COVERAGE FOR REINFORCEMENT:
Center Line (unless otherwise noted)
s 3" to ties

CING STEEL:
reinforcing steel contractor shall fabricate all reinforcement and furnish all
ssories, chairs, spacer bars and supports necessary to secure the reinforcement
ss shown otherwise on the plans and details.
ncrete reinforcement shall be placed according to the CRSI "Recommended
ctices for Placing Reinforcing Bars".
mpression and tension lap splices for cast-in-place concrete shall be 36 bar
meters minimum, unless noted otherwise.
sion lap splices for reinforced masonry shall be 40 bar diameters minimum,
ss noted otherwise.
izontal reinforcing steel in footings and concrete walls shall be continuous
nd corners.
laps in WWF should be one mesh plus two inches at splices.
vide two #5 reinforcing bars each side around openings in concrete walls and
e. Bars shall extend 24" beyond the corners of the openings. Also provide 2 -
diagonal bars at each corner of the opening.
nforcing bars shall not be welded without the approval of the structural engineer.
d welding of reinforcing steel shall be performed by welders specifically certified
reinforcing steel. Prior to welding the "carbon equivalent" (CE) of steel shall be
rmined. Reinforcing steel whose "CE" can't be identified or whose "CE" exceeds
5% shall not be welded. Except for reinforcing steel conforming to ASTM A-706
forcing steel shall be preheated as shown in table 1.RGA 3-77. In addition, steel
a "CE" between 0.66% and 0.75% shall be welded only when prior qualification
verify acceptable weldability.

ED MASONRY:
low load bearing masonry units shall conform to ASTM C90 Grade N Type I, in
ition to the requirements of the quality control standards of the Concrete/Masonry
ociation, minimum required compressive strength of block units shall be 2800 psi
ed on net area)
sonry Units shall have been cured for not less than 28 days when placed in the
cture.
use of admixtures will not be permitted in the grout or mortar unless substantial-
data is submitted to and approved by the structural engineer or the architect.
ut for vertically reinforced masonry walls and bond beams shall have fc = 3,000
in 28 days and shall have a slump of 9" to 10".
masonry walls shall have horizontal reinforcing consisting of galvanized standard
ght 9 ga. "Dur-O-Wall" or equal. All reinforcing shall be located every other course
ss noted otherwise.
ots as shown shall match the size and number of footing reinforcing unless noted
rwise. Hook into footing 12" minimum and lap 30 diameters with main steel.
e "S" is required for bearing walls below grade. Type "N" mortar is required for
ring walls above grade. Mortar shall be of a portland cement type mix.
Architectural Plan for location and detail of vertical control joints.
imum spacing for control joints in masonry walls shall not exceed 24'-0" o.c.
steel beams bearing on masonry shall have two cores minimum filled with grout
tly below the bearing point, except as noted on the plans.
d beams 8" or smaller use one #5 continuous. Bond beams 10" or larger use 2 -
ntinuous unless noted otherwise.
sonry prisms shall be performed in accordance with ASTM E447-84. Prepare 1
f 5 prisms prior to the start of construction. Test 2 of these a 7 days and the
ining 3 at 28 days. Prepare 1 additional set of three prisms for every 5000 sf
nstruction. Test 1 of these at 7 days and the remaining 2 at 28 days. Refer to
cial Inspections" in the structural notes and to the project specifications for
ional information.

NEERED WOOD TRUSSES:
l lumber for wood trusses shall be in accordance with manufacturer's written recommendations
ood trusses shall have bridging and bracing in accordance with manufacturer's requirements.
ood trusses shall be designed for a top chord live load as noted under Design Live Loads plus
ow load/drift in accordance with applicable building codes (minimum top chord dead load shall
10 psf, minimum bottom chord dead load shall be 10 psf.)
e load deflection limitations – roof trusses less than L/360.
bmit complete shop drawings for approval, showing the erection plan, all bearing conditions,
d connections. Calculations certified by a Professional Engineer will be required for all wood
sses, connections and bearing attachments/components. (Submit certified plans and calcula-
ns to the local building official).
ood truss supplier shall detail end bearing of trusses so as not to exceed perpendicular to grain
ding of wood plates that support the trusses.

proved by the Engineer.
d. All nailing shall conform to the nailing schedule unless noted otherwise.
e. All sills and plates resting on concrete or masonry, which is in contact with the earth or resting on foundations shall be pressure treated Southern Pine No. 2.
f. All bolt heads and nuts bearing on wood shall have standard cut washers. All bolt holes in wood shall be drilled 1/32" larger in diameter than nominal bolt diameters.
g. Bolts in wood shall not be less than 7 diameters from the end & 4 diameters from edge of member.
h. All framing anchors, post caps, bases, hangers, straps, etc., shall be as manufactured by "Simpson" Company or approved equal.
i. Top plates of all wood stud walls to be 2 – 2x (same width as studs), lap 48" (min.) with not less than 6-16d nails at each lap and not more than 16" between nails. Splice at studs only.
j. Moisture content of wood at time of placing shall not exceed 19%.
k. All member sizes given on the drawings are nominal sizes.
l. Spacing of bridging for joists shall not exceed 8'-0".
m. Wood lintels and headers shall have a full 3" length of bearing at each end (min.).
n. Double all joists under parallel partitions.
o. All beams and joists not bearing on supporting members shall be framed with "Simpson HHUS" joist hangers or equal. Use type as required for the application shown on the drawings.
p. Wood joists shall bear the full width of supporting members (stud walls, beams, etc.) unless otherwise indicated.
q. Wood beams made of 2 or more 2x's shall be nailed together with 3 rows of 16d nails at 12" o.c. For a 3 piece member, install the specified nailing on each side.
r. Unless noted, 4" wide stud walls are to have 2 x 4 studs at 16" o.c.
s. Unless noted, exterior walls are to be 2 x 6 studs at 16" o.c.
t. Sill plates to be bolted to foundation walls with 5/8" round bolts at 6'-0" o.c. maximum, bolts to Extend 15" minimum into grouted masonry. Each sill plate to have a minimum of 2 bolts with one bolt located within 12" of each end of each plate.

11. NAILING SCHEDULE:
a. joists or rafters to sides of studs
8" joist or less 3 -16d
for each additional 4" in depth of joist 1 – 16d
b. bridging to joists, toenail each end 2 – 8d
blocking between joists or rafters to joists or rafters – toenail each side, each end 2 – 10d
blocking between studs each end 2 – 10d toenails or 2 – 16d
c. 1" x 6" subfloor to each joist, face nail 2 – 8d
d. wider than 1" x 6" to each joist, face nail 3 – 8d
e. 2" subfloor to joist or girder, blind and face nail 2 – 16d
f. Sole plate to joist or blocking, face nail 16d @ 16" o.c.
g. Top plate to stud, end nail 2 – 16d
h. Stud to sole plate 4 – 10d toenail, or 2 – 16d end nail
i. Double top plates, face nail 16d @ 16" o.c.
j. Continuous header, two pieces 16d @ 12" o.c. along each edge
k. Ceiling joists to plate, toenail 3 – 8d
l. Continuous header to studs, toenail 4 – 8d
m. Ceiling joists, laps over partitions, face nail 3 – 16d
n. Ceiling joists to parallel rafters, face nail 3 – 16d
o. Joists or rafters at all bearing, toenail each side 2 -10d
p. 1" brace to each stud and plate, face nail 2- 8d
q. 1" x 8" sheathing or less to each bearing, face nail 2 – 8d
r. wider than 1" x 8" sheathing to each bearing, face nail 3- 8d
s. Built-up corner studs 16d @ 24" o.c.
t. Double studs 10d @ 12" o.c. direct
u. Rim joists 6" or less 16d (2/ joist) end nail
v. Rim joists 6" or more 16d (3/ joist) end nail
w. Diagonal bracing (to stud & plate) 16d (3/ joist) end nail

12. BACKFILLING:
a. No backfilling and compacting of earth shall be permitted against foundation walls until supporting slabs have been poured and have reached 75% of their design strength or unless adequate bracing submitted for review has been approved.
b. Both sides of foundation walls shall be backfilled simultaneously so as to prevent overturning or lateral movement of walls.
c. All grade beams shall be adequately braced to prevent lateral movement during backfilling and compaction.

13. CONSTRUCTION AND CONTROL JOINTS:
a. Construction joints shall be made as detailed on the drawings.
b. Maximum spacing for control joints shall be 15'-0".
c. A 15'-0" maximum spacing of control joints may not insure complete control of shrinkage cracks. A closer spacing may be used by request of the contractor if more complete shrinkage crack control is desired. Contractor to verify with Owner.

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a. All expansion bolts shall be KWIK bolts or REDHEAD as noted on the plans. Minimum embedment unless otherwise noted shall be 4" for 1/2" diameter bolts, and 5" for 5/8" and 3/4" diameter bolts.

15. CONSTRUCTION PROCEDURES:
a. The structure shall be adequately braced and shored against wind and construction loads during erection. Structural members are designed for "in-place" loads only.
b. Comply with all applicable city, state and federal laws, including Occupational Safety and Health Administration Act (OSHA) and regulations adopted pursuant thereto.
c. The contract structural drawings and specifications represent the finished structure. Unless otherwise noted, they do not indicate the means or method of construction. Provide all measures necessary to protect the structure, workmen, or other persons during construction. Such measures shall include, but are not limited to, bracing, shoring for construction equipment, shoring for earth loads, forms, scaffolding, planking, safety nets, support and bracing for cranes and gin poles, etc.
d. Engage properly qualified persons to determine where and how temporary precautionary measures shall be used and inspect same in the field. Observation visits to the jobsite by the ENGINEER'S representative shall not include inspection of the above items.
e. Supervise and direct the work so as to maintain sole responsibility for all construction means, methods, techniques, sequences, and procedures. As a part of this responsibility, retain the services of a licensed structural engineer to design and supervise any scaffolding for workmen, and all shoring of forms and elements of the construction.

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a. The contractors shall verify all dimensions and elevations with the architectural drawings. Where discrepancies occur, it is the contractors responsibility to notify the Architect prior to proceeding with construction.

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a. The contractors shall verify, by field check, all sizes, dimensions, elevations, locations, etc. of elements of the existing construction which are relative to the new construction.
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a. Shop drawings, unless otherwise noted, shall be submitted on reproducible transparencies with one print for review prior to fabrication. Send reproducible transparencies and print in a mailing tube. Transparencies which will not produce a legible print shall be returned for resubmittal.
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f. Turn around time for shop drawings shall be two weeks from the date received in the engineers office.

Jamb architect
P.O. Box 340 Forest Lake, Minnesota 55

I hereby certify that this plan, specification, or report was prepared by me or under my direct supervision and that I am a duly Registered Professional Engineer under the laws of the State of Minnesota.

1st CHURCH of CHRIST SCIENTIST
CHURCH ROOF RE-DESIGN & REPLACEMENT

Project No. 0810
Date: 9/18/08
Revisions:

Sheet No. S-1

THE MINNESOTA BOARD OF ARCHITECTURE, ENGINEERING, LAND SURVEYING,
LANDSCAPE ARCHITECTURE, GEOSCIENCE & INTERIOR DESIGN

July 8, 2009

CONFIDENTIAL
By Certified U.S. Mail
Return Receipt Requested
Number 7007 0710 0001 3781 8556

Mr. Jerry Anderson
JAMB Architects
Post Office Box 310
Forest Lake, Minnesota 55025

RE: Jerry Anderson, *File No. 2009-0029*

Dear Mr. Anderson:

Please be informed that the Minnesota Board of Architecture, Engineering, Land Surveying, Landscape Architecture, Geoscience, and Interior Design ("Board") is continuing its investigation in the above referenced matter. *Originally, it had come to the attention of the Board that you may have practiced architecture without a license and held yourself out as an Architect in the State of Minnesota, by signing the drawings for the Church Roof Re-Design & Replacement project for 1st Church of Christ Scientist on September 18, 2008, during the time your Architect license had lapsed. The alleged violations were for Minnesota Statutes section 326.02, subdivisions 1 and 2 (2008). In addition to the practice of architecture and holding yourself out as an Architect during the time your license lapsed, it is further alleged that you may not have conducted yourself properly as an Architect and that you may have practiced professional engineering without a license by preparing or having drawn the drawings identified as sheet S-1, on 09/18/008, for the 1st Church of Christ Scientist, Church Roof Re-Design & Replacement, of Saint Paul, Minnesota project (See Exhibit A enclosed) and that your actions may be violations of Minnesota Statutes section 326.02, subdivision 3, Minnesota Statutes section 326.03, subdivision 1, Minnesota Statutes section 326.111, subdivisions 1 and 4 (a) (1), (2), and (3) (2008) and Minnesota Rules 1805.0100, Minnesota Rules 1805.0200, subparts 1 and subparts 4 (C), and (D) (2007).*

85 East 7th Place, Suite 160, St. Paul, MN 55101
p. 651.296.2388 ■ f. 651.297.5310 ■ TTY 800.627.3529
www.aelslagid.state.mn.us

FILE COPY
EXHIBIT D

The purpose of this investigation is to determine whether or not the facts alleged are true and, if so, whether enforcement action should be initiated pursuant to Minnesota law. This investigation is authorized by Minnesota Statutes section 214.10 (2008) and Minnesota Statutes section 326.111 (2008). Enclosed are the statutes and rules the Board is empowered to enforce.

Please provide a detailed written response to these allegations on or before Wednesday, July 22, 2009. The Board specifically requests that the following information or documents be included in your written response:

a. Did you prepare/draw the plans identified as sheet S-1, dated 09/18/08, for the 1st Church of Christ Scientist – Church Roof Re-design & Replacement project in St. Paul, MN? (See enclosed Exhibit A).
b. If you prepared/drew the plans identified as sheet S-1, dated 09/18/08, for the for the 1st Church of Christ Scientist – Church Roof Re-design & Replacement project in St. Paul, MN (See enclosed Exhibit A), did a licensed Minnesota Professional Engineer review them? If so, please specify the name and license number of the licensed Minnesota Professional Engineer who reviewed these particular plans. Also, specify the date when you prepared/drew the plans and the date the licensed Professional Engineer reviewed them.
c. Did [redacted] prepare/draw the plans identified as sheet S-1, dated 09/18/08, for the for the 1st Church of Christ Scientist – Church Roof Re-design & Replacement project in St. Paul, MN (See enclosed Exhibit A)? If not, please provide the name, address, and phone number of the person who did prepare/draw these plans. If you drew the plans, did you and [redacted] have an agreement or understanding that you would draw the plans instead of [redacted]?
d. Did [redacted] personally sign and certify the drawings as a Professional Engineer, on the plans/drawings identified as sheet S-1, dated 09/18/08, for the for the 1st Church of Christ Scientist – Church Roof Re-design & Replacement project in St. Paul, MN (See enclosed Exhibit A)? If so, state the date when [redacted] did so. If not, please provide the name, address and phone number of the person who signed and certified them, the date when the signing and certifying was done, and state whether the signature was handwritten or made using a stamp of [redacted] signature.
e. Did [redacted] give you his permission to use his Professional Engineer's certification on the drawings, for the 1st Church of Christ Scientist – Church Roof

Re-design & Replacement project in St. Paul, MN (See enclosed Exhibit A)? Did [redacted] give you his permission to sign his name on the drawings, for the 1st Church of Christ Scientist-Church Roof Re-design & Replacement project in St. Paul, MN (See enclosed Exhibit A), and if so, was the permission for you to place a handwritten signature or use a stamp of [redacted] signature?

f. Did you and [redacted] have an agreement or an understanding that you could prepare or draw the "Roof Framing Plan", identified as sheet S-1, dated 09/18/08, for the 1st Church of Christ – Church Roof Re-design & Replacement project in St. Paul, MN (see enclosed Exhibit A) and that [redacted] would review, sign, and certify them? If you prepared the drawings on the original of enclosed Exhibit A, did [redacted] actually review the drawings and, if so, when?

g. Did you and [redacted] have an agreement or an understanding that you could prepare 'structural design plans' on projects other than the 1st Church of Christ Scientist-Church Roof Re-design & Replacement project, and that [redacted] would review them? Did you and [redacted] have an agreement or an understanding that you could prepare 'structural design plans' on projects other than the 1st Church of Christ Scientist-Church Roof Re-design & Replacement project, and that you had [redacted] permission to use his Professional Engineer's certification and place his signature (handwritten or stamped) on the drawings you prepared as it relates to the practice of professional engineering?

h. Did you pay [redacted] any money or other form of compensation in exchange for his permission to use his Professional Engineer's certification and/or place his signature on the drawings identified as sheet S-1, dated 09/18/08, for the for the 1st Church of Christ Scientist – Church Roof Re-design & Replacement project in St. Paul, MN (see enclosed Exhibit A)? If so, how much money did you pay him ? If a form of compensation other than money was provided to [redacted] state specifically the nature and extent/amount of the compensation.

Please include File Number 2009-0029 on all correspondence. Of course, you may include any additional information or documents that you believe will explain your position.

Information provided to the Board or to the Office of the Attorney General as part of an active investigation is confidential pursuant to the Minnesota Government Data Practices Act, Minnesota Statutes section 13.41 (2006). Such information is for the use of the Board and the Attorney General in evaluating the complaint. In accordance with

statutes, rules, and professional standards governing legal action, information provided to the Board and the Attorney General may, in some circumstances, be disclosed to certain other persons or entities, including but not limited to the Office of Administrative Hearings and appellate courts. Thus, this information may thereby become public data. Please read the enclosed Tennessen Warning.

Your cooperation is requested. Should you have any questions, please contact me by dialing (651) 757-1510.

Sincerely,

Lynette DuFresne
Investigator

Enclosure: Minnesota Statues 326.02 – 326.15 (2008)
Minnesota Rules 1800 and 1805 (2007)
Exhibit A
Tennessen Warning
Filing of a Complaint Brochure

A. Prefinished metal flashing and keepers (copper to match existing)

B. Prefinished gutters and downspouts (copper to match existing)

C. Existing Brick

D. New Brick (to match existing)

E. New or existing trim - painted (to match existing)

F. Prefinished aluminum louver vents (w/ insect screens)

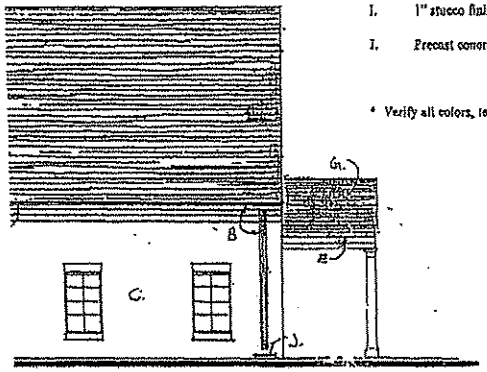
G. 340# fiberglass reinforced asphalt shingles

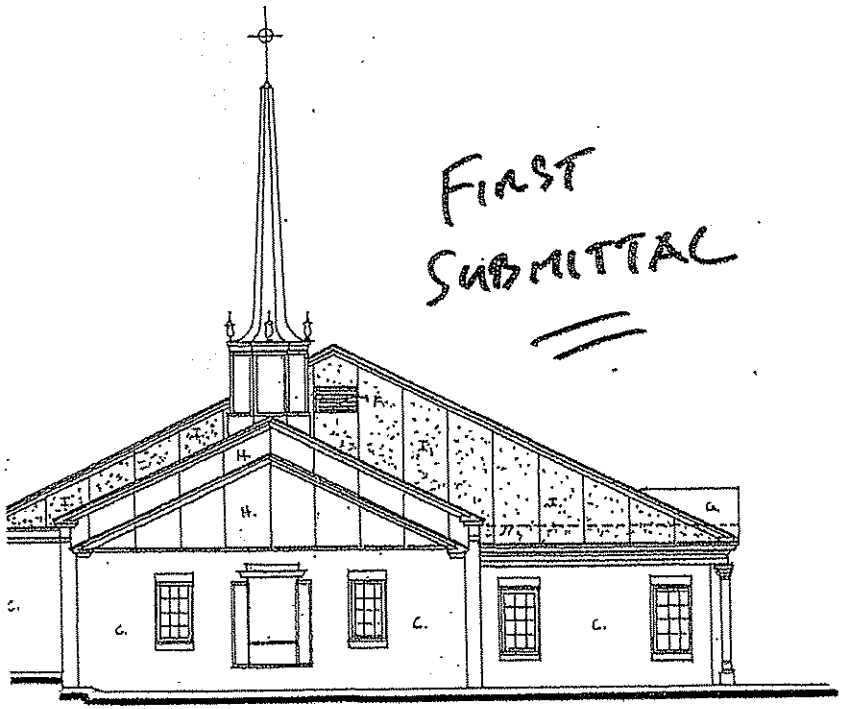
H. Existing stucco finish

I. 1" stucco finish (texture & color to match existing stucco)

J. Precast concrete splashblocks

* Verify all colors, textures, finishes and material selections with Owner prior to application





FIRST SUBMITTAL

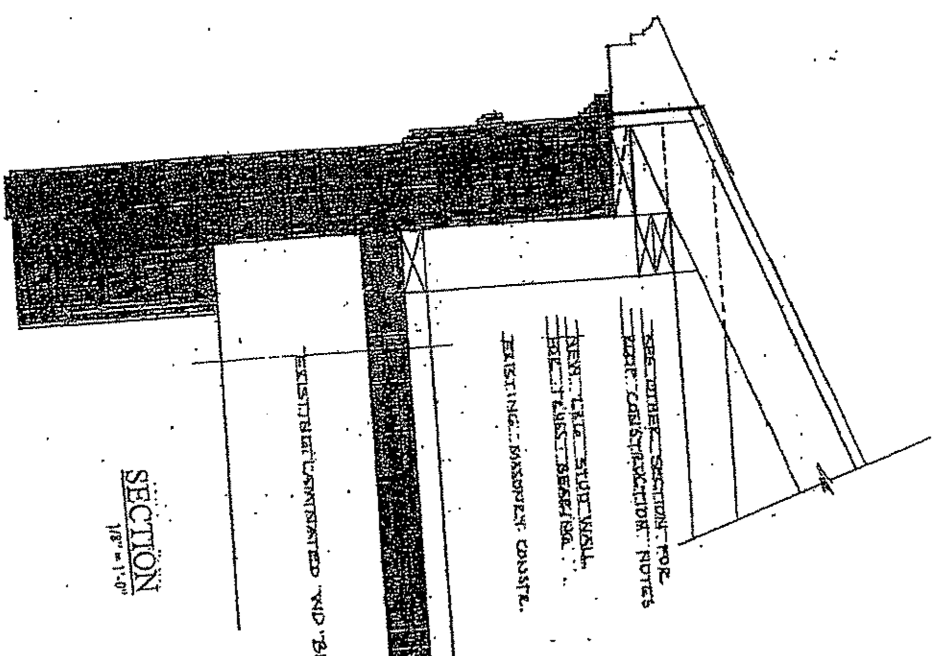
WEST - ELEVATION
1/8" = 1'-0"

Jamb architec
P.O. Box 310 Forest Lake, Minnesota 5
(612) 670-4

1st CHURCH of CHRIST SCIENTIST
CHURCH ROOF RE-DESIGN & REPLACEMENT
ST. PAUL MINNESOTA

Project No. 0810
Date 9/18/08
Revisions

Sheet No. A-1



SECTION
1/8" = 1'-0"

1st CHURCH of CHRIST SCIENTIST	Project No. 0810
CHURCH ROOF RE-DESIGN & REPLACEMENT	Date 9/18/08
ST. PAUL MINNESOTA	Revisions
	Sheet No. A-2

jamb arc
P.O. Box 310 Forest Lake

SOIL [illegible] DESIGN:
[illegible] Building Code of State of Minnesota Appendix

[illegible] shall bear on natural undisturbed soil or on compacted granular fill.
[illegible] for subsurface soils in the footing areas referred to in the soil
[illegible] above shall be strictly followed.
[illegible] of the footings as detailed is of questionable bearing value, the
[illegible] office shall be notified at once.
[illegible] changes shall be stepped at a ratio of 1 (vert) to 2 (horiz).
[illegible] vertical step shall be 1'-4" unless noted otherwise.
[illegible] footings shall have a minimum soil cover of 3'-6" measured from
the footing unless otherwise noted.
[illegible] for anticipated settlement values. The structural engineer shall verify
[illegible] criteria will not be detrimental to the building or its operations.

[illegible], corridors and stairs	100 psf
[illegible]	10 psf + 30 psf
	125 psf
	40 psf
	90 mph
	Material Self Weights

[illegible]
weight if heavier
and/or sliding snow

[illegible]	Type Mix	Location
[illegible]	Std. Weight	Interior slabs and walls
	Std. Weight	Exterior slabs and walls
	Air Entrained	
	Std. Weight	Footings
	Std. Weight	Interior Topping
	f'm = 1,500 psi (28 days)	
	Fy = 60,000 psi	ASTM A615
[illegible]	Fy = 36,000 psi	ASTM A36
[illegible]	Fy = 46,000 psi	ASTM A500, Gr. B
[illegible]	Fy = 35,000 psi	ASTM A53, Gr. B
[illegible]	Fy = 36,000 psi	ASTM A36
	Fu = 120,000 psi	ASTM A325
	Fu = 60,000 psi	ASTM A307
[illegible]	Fy = 70,000 psi	ASTM A185
[illegible] Fabric		

COVERAGE FOR REINFORCEMENT:
1"
3"
Center of Slab
Center of Wall

COVERAGE FOR REINFORCEMENT:
Center Line (unless otherwise noted)
1" to slot

STEEL:
[illegible] steel contractor shall fabricate all reinforcement and furnish all
[illegible], chairs, spacer bars and supports necessary to secure the reinforcement
[illegible] otherwise on the plans and details.
[illegible] reinforcement shall be placed according to the CRSI "Recommended
[illegible] for Placing Reinforcing Bars".
[illegible] and tension lap splices for cast-in-place concrete shall be 36 bar
[illegible] minimum, unless noted otherwise.
[illegible] splices for reinforced masonry shall be 48 bar diameters minimum,
[illegible] otherwise.
[illegible] reinforcing steel in footings and concrete walls shall be continuous
[illegible].
[illegible] WWF should be one mesh plus two inches at splices.
[illegible] #5 reinforcing bars each side around openings in concrete walls and
[illegible] shall extend 24" beyond the corners of the openings. Also provide 2 -
[illegible] bars at each corner of the opening.
[illegible] bars shall not be welded without the approval of the structural engineer.
[illegible] of reinforcing steel shall be performed by welders specifically certified
[illegible] steel. Prior to welding the "carbon equivalent" (CE) of steel shall be
[illegible]. Reinforcing steel whose "CE" can't be identified or whose "CE" exceeds
[illegible] shall not be welded. Except for reinforcing steel conforming to ASTM A706
[illegible] steel shall be preheated as shown in table 1.RGA 2-77. In addition, steel
[illegible] CE between 0.45% and 0.75% shall be welded only when prior qualification
[illegible] acceptable weldability.

MASONRY:
[illegible] load bearing masonry units shall conform to ASTM C90 Grade N Type I, in
[illegible] the requirements of the quality control standards of the Concrete Masonry
[illegible] minimum required compressive strength of block units shall be 2100 psi
[illegible] net area)
[illegible] Units shall have been cured for not less than 28 days when placed in the
[illegible].
[illegible] of admixtures will not be permitted in the grout or mortar unless substantia-
[illegible] is submitted to and approved by the structural engineer or the architect.
[illegible] for vertically reinforced masonry walls and bond beams shall have f'c = 3,000
[illegible] 28 days and shall have a slump of 8" to 10".
[illegible] masonry walls shall have horizontal reinforcing consisting of galvanized standard
[illegible] 9 ga. "Dur-O-Wall" or equal. All reinforcing shall be located every other course
[illegible] noted otherwise.
[illegible] as shown shall match the size and number of footing reinforcing unless noted
[illegible]. Hook into footing 12" minimum and lap 30 diameters with main steel.
[illegible] "S" is required for bearing walls below grade. Type "N" mortar is required for
[illegible] walls above grade. Mortar shall be of a portland cement type mix.
[illegible] Architectural Plan for location and detail of vertical control joints.
[illegible] spacing for control joints in masonry walls shall not exceed 24'-0" o.c.
[illegible] beams bearing on masonry shall have two course minimum filled with grout
[illegible] below the bearing point, except as noted on the plans.
[illegible] 8" or smaller use one #5 continuous. Bond beams 10" or larger use 2 -
[illegible] unless noted otherwise.
[illegible] prism shall be performed in accordance with ASTM E447-84. Prepare 3
[illegible] prisms prior to the start of construction. Test 2 of these at 7 days and the
[illegible] 1 at 28 days. Prepare 3 additional set of three prisms for every 5000 sf
[illegible] 2 at 28 days. Prepare 1 additional ... and the remaining 2 at 28 days. Refer to
[illegible]. Test 1 of these at 7 days and the remaining ... Refer to
[illegible] Inspections" in the structural notes and in the project specifications for
[illegible] information.

ENGINEERED WOOD TRUSSES:
[illegible] for wood trusses shall be in accordance with manufacturer's written recommendations
[illegible] trusses shall have bridging and bracing in accordance with manufacturer's requirements.
[illegible] trusses shall be designed for a top chord live load as noted under Design Live Loads plus
[illegible] load in accordance with applicable building codes (minimum top chord dead load shall
[illegible] 10 psf minimum bottom chord dead load shall be 10 psf.)
[illegible] deflection limitations - roof trusses less than L/360.
[illegible] complete shop drawings for approval, showing the erection plan, all bearing conditions,
[illegible] connections. Calculations certified by a Professional Engineer will be required for all wood
[illegible], connections and bearing attachments/components. (Submit certified plans and calcula-
[illegible] to the local building official).
[illegible] truss supplier shall detail end bearing of trusses so as not to exceed perpendicular to grain
[illegible] of wood plates that support the trusses.

e. [illegible] shall be pressure treated Southern Pine No. 2.
f. All bolt heads and nuts bearing on wood shall have standard cut washers. All bolt holes in wood on foundations shall be drilled 1/32" larger in diameter than nominal bolt diameters.
g. Bolts in wood shall not be less than 7 diameters from the end & 4 diameters from edge of member.
h. All framing anchors, post caps, bases, hangers, straps, etc. shall be as manufactured by "Simpson" Company or approved equal.
i. Top plates of all wood stud walls to be 2 - 2x (same width as studs), lap 48" (min.) with not less than 8-16d nails at each lap and not more than 16" between nails. Splice at studs only.
j. Moisture content of wood at time of placing shall not exceed 19%.
k. All member sizes given on the drawings are nominal sizes.
l. Spacing of bridging for joists shall not exceed 8'-0".
m. Wood lintels and headers shall have a full 3" length of bearing at each end (min.).
n. Double all joists under parallel partitions.
o. All beams and joists not bearing on supporting members shall be framed with "Simpson HHUS" joist hangers or equal. Use type as required for the application shown on the drawings.
p. Wood joists shall bear the full width of supporting members (stud walls, beams, etc.) unless otherwise indicated.
q. Wood beams made of 2 or more 2x's shall be nailed together with 2 rows of 16d nails at 12" o.c. For a 3 piece member, install the specified nailing on each side.
r. Unless noted, 4" wide stud walls are to have 2 x 4 studs at 16" o.c.
s. Unless noted, exterior walls are to be 2 x 6 studs at 16" o.c.
t. Sill plates to be bolted to foundation walls with 5/8" round bolts at 6'-0" o.c. maximum, bolts to extend 12" minimum into grouted masonry. Each sill plate to have a minimum of 2 bolts with one bolt located within 12" of each end of each plate.

11. NAILING SCHEDULE:
a. Joists or rafters to sides of studs
8" joist or less 2 - 16d
for each additional 4" in depth of joist 1 - 16d
b. bridging to joists, toenail each end 2 - 8d
blocking between joists or rafters to joists or rafters - toenail each side, each end 2 - 10d
blocking between studs each end 2 - 10d toenails or 2 - 16d
c. 1" x 6" subfloor to each joist, face nail 2 - 8d
d. wider than 1" x 6" to each joist, face nail 3 - 8d
e. 2" subfloor to joist or girder, blind and face nail 2 - 16d
f. Sole plate to joist or blocking, face nail 16d @ 16" o.c.
g. Top plate to stud, end nail 2 - 16d
h. Stud to sole plate 4 - 10d toenail, or 2 - 16d end nail
i. Double top plates, face nail 16d @ 16" o.c.
j. Continuous header, two pieces 16d @ 12" o.c. along each edge
k. Ceiling joists to plate, toenail 3 - 8d
l. Continuous header to studs, toenail 4 - 8d
m. Ceiling joists, laps over partitions, face nail 3 - 16d
n. Ceiling joists to parallel rafters, face nail 3 - 16d
o. Joists or rafters at all bearing, toenail each side 2 - 10d
p. 1" brace to each stud and plate, face nail 2 - 8d
q. 1" x 8" sheathing or less to each bearing, face nail 2 - 8d
r. wider than 1" x 8" sheathing to each bearing, face nail 3 - 8d
s. Built-up corner studs 16d @ 24" o.c.
t. Double studs 10d @ 12" o.c. direct
u. Rim joists 6" or less 16d (2/joist) end nail
v. Rim joists 8" or more 16d (3/joist) end nail
w. Diagonal bracing (to stud & plate) 16d (2/joist) end nail

12. BACKFILLING:
a. No backfilling and compacting of earth shall be permitted against foundation walls until supporting slabs have been poured and have reached 75% of their design strength or unless adequate bracing submitted for review has been approved.
b. Both sides of foundation walls shall be backfilled simultaneously so as to prevent overturning or lateral movement of walls.
c. All grade beams shall be adequately braced to prevent lateral movement during backfilling and compaction.

13. CONSTRUCTION AND CONTROL JOINTS:
a. Construction joints shall be made as detailed on the drawings.
b. Maximum spacing for control joints shall be 15'-0".
c. A 15'-0" maximum spacing of control joints may not insure complete control of shrinkage cracks. A closer spacing may be used by request of the contractor if more complete shrinkage crack control is desired. Contractor to verify with Owner.

14. EXPANSION BOLTS:
a. All expansion bolts shall be KWIK bolts or REDHEAD as noted on the plans. Minimum embedment unless otherwise noted shall be 5" for 1/2" diameter bolts, and 3" for 5/8" and 3/4" diameter bolts.

15. CONSTRUCTION PROCEDURES:
a. The structure shall be adequately braced and shored against wind and construction loads during erection. Structural members are designed for "in-place" loads only.
b. Comply with all applicable city, state and federal laws, including Occupational Safety and Health Administration Act (OSHA) and regulations adopted pursuant thereto.
c. The contract structural drawings and specifications represent the finished structure. Unless otherwise noted, they do not indicate the means or method of construction. Provide all measures necessary to protect the structure, workmen, or other persons during construction. Such measures shall include, but are not limited to, bracing, shoring for construction equipment, shoring for earth banks, forms, scaffolding, planking, safety nets, support and bracing for cranes and gin poles, etc.
d. Engage properly qualified persons to determine where and how temporary precautionary measures shall be used and inspect same in the field. Observation visits to the jobsite by the ENGINEER'S representative shall not include inspection of the above items.
e. Supervise and direct the work so as to maintain sole responsibility for all construction means, methods, techniques, sequences, and procedures. As a part of this responsibility, retain the services of a licensed Structural engineer to design and supervise any scaffolding for workmen, and all shoring of forms and elements of the construction.

16. COORDINATION WITH ARCHITECTURAL DRAWINGS:
a. The contractor shall verify all dimensions and elevations with the architectural drawings. Where discrepancies occur, it is the contractor's responsibility to notify the Architect prior to proceeding with construction.

17. NEW WORK IN CONNECTION WITH EXISTING CONSTRUCTION:
a. The contractor shall verify, by field check, all sizes, dimensions, elevations, locations, etc. of elements of the existing construction which are relative to the new construction.
b. All dimensions involving new work tying into or governed by existing construction shall be field checked by the contractor and furnished to the subcontractor prior to fabrication of any work. The verified dimensions shall appear and be noted as such on the first shop drawings submitted.
c. Cutting of existing structural steel is prohibited without approval of the engineer.

18. SHOP DRAWINGS:
a. Shop drawings, unless otherwise noted, shall be submitted on reproducible transparencies with one print for review prior to fabrication. Send reproducible transparencies and print in a mailing tube. Transparencies which will not produce a legible print shall be returned for resubmittal.
b. Shop drawings are to be prepared under the supervision of a registered professional engineer. They are to include complete details and schedules for fabrication and assembly of structural members, procedures and diagrams.
c. Fabricators shall draw their own erection plans. Copying the structural plans and using them as erection plans is not acceptable.
d. Prior to submittal, the contractor shall review the shop drawings and make any corrections required. The contractor shall stamp and sign the drawings as evidence that they have reviewed them.
e. Shop drawings shall be submitted for all structural components.
f. Turn around time for shop drawings shall be two weeks from the date received in the engineers office.

lamb archit
P.O. Box 310 Forest Lake, Minnes
(612

I hereby certify that this plan, specification, or report was prepared by me or under my direct supervision and that I am a duly Registered Professional Engineer under the laws of the State of [illegible]
[redacted]
Date 9/18/08 Reg. No. [illegible]

1st CHURCH of CHRIST SCIENTIST
CHURCH ROOF RE-DESIGN & REPLACEMENT
ST. PAUL MINNESOTA

Project No. 0810
Date: 9/18/08
Revisions:
Sheet No. S-1

A. Prefinished metal flashing and keepers (copper to match existing)

B. Prefinished gutters and downspouts (copper to match existing)

C. Existing Brick

D. New Brick (to match existing)

E. New or existing trim - painted (to match existing)

F. Prefinished aluminum louver vents (w/ insect screens)

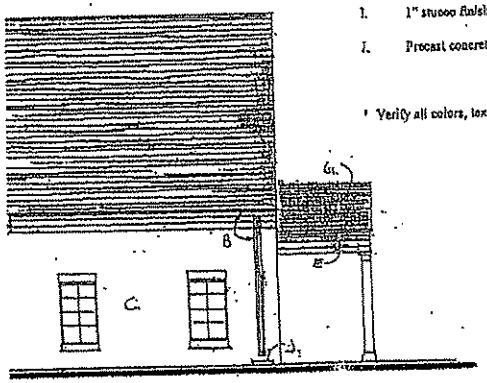
G. 340# fiberglass reinforced asphalt shingles

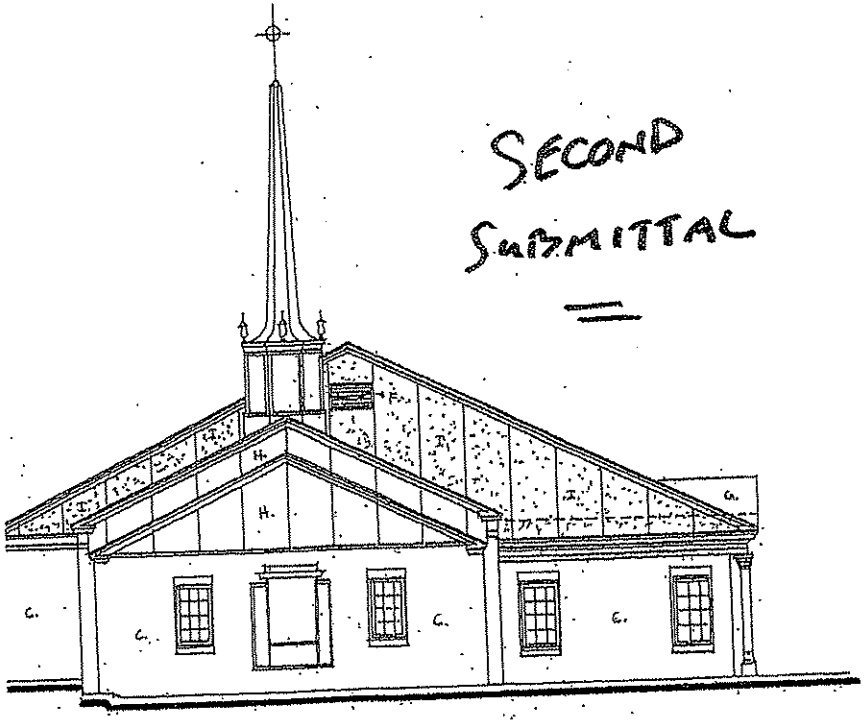
H. Existing stucco finish

I. 1" stucco finish (texture & color to match existing stucco)

J. Precast concrete splashblocks

* Verify all colors, textures, finishes and material selections with Owner prior to application





SECOND SUBMITTAL

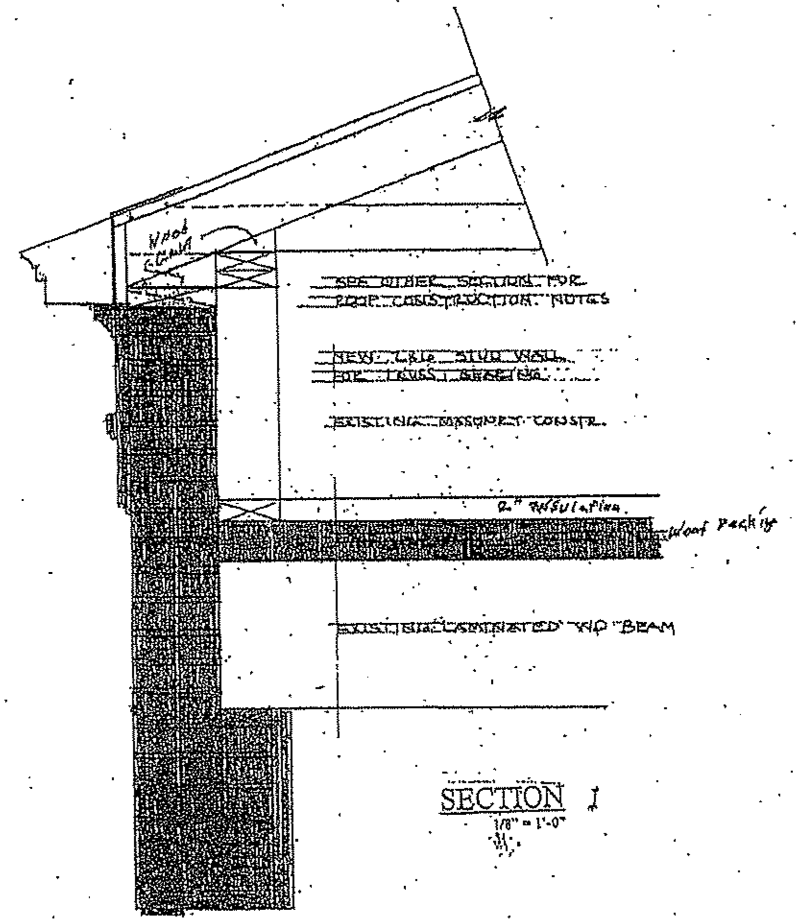
WEST ELEVATION
1/8" = 1'-0"

Jlamb archi
P.O. Box 310 Forest Lake, Minnesota (612)

1st CHURCH of CHRIST SCIENTIST
CHURCH ROOF RE-DESIGN & REPLACEMENT
ST. PAUL MINNESOTA

Project No. 0810
Date 9/10/08
Revisions:

Sheet No. A-1



SECTION 1
1/8" = 1'-0"

Wall Footing

Roof - 32.7 x 55 = 1799
12" Blk. Wall - 14 x 100 = 1400
Conc. Ftg. x 150 x 1.75 = 263

3462 #

3462 ÷ 2000 = 1.73' OK
∴ Ftg. Size = 1.75'

1st CHURCH of CHRIST SCIENTIST
CHURCH ROOF RE-DESIGN & REPLACEMENT

lamb arch
P.O. Box 393, Forest Lake, Minnesota

Project No. 0810
Date: 9/18/08
Revisions:
Sheet No. A-2

bear on natural undisturbed soil or on compacted granular fill.
...bearing soils in the footing areas referred to in the soil ...
...re shall be strictly followed.
...the footings as detailed is of questionable bearing value, the ...
...ffice shall be notified at once.
... changes shall be stepped at a ratio of 1 (vert) to 2 (horiz).
... lap shall be 1'-4" unless noted otherwise.
...ngs shall have a minimum soil cover of 3'-6" measured from ...
...ing unless otherwise noted.
...depicted resistance values. The structural engineer shall verify ...
...tests will not be detrimental to the building or its operation.

...floors and stairs	100 psf
	*10 psf + 30 psf
	125 psf
	**40 psf
	90 mph
	Material Self Weight

...Elevator
...Bag snow

Type Mix	Location
Std. Weight	Interior slabs and walls
Std. Weight	Exterior slabs and walls
Air Entrained	
Std. Weight	Footings
Std. Weight	Interior Topping

f'm = 1,500 psi (28 days)	
Fy = 60,000 psi	ASTM A615
Fy = 36,000 psi	ASTM A36
Fy = 46,000 psi	ASTM A500, Grd. B
Fy = 36,000 psi	ASTM A53, Grd. B
Fy = 36,000 psi	ASTM A36
Fu = 120,000 psi	ASTM A325
Fu = 60,000 psi	ASTM A307
Fy = 70,000 psi	
	ASTM A185

...OR REINFORCEMENT:
1"
2"
Center of Slab
Center of Wall

...Y REINFORCEMENT:
Center Line (unless otherwise noted)
3" to ties

...ractor shall fabricate all reinforcement and furnish all ...
...air bars and supports necessary to secure the reinforcement ...
...n on the plans and details.
...nt shall be placed according to the CRSI "Recommended Reinforcing Bars".
...lap splices for cast-in-place concrete shall be 36 bar ...
...unless noted otherwise.
...reinforced masonry shall be 48 bar diameters minimum.
...
...steel in footings and concrete walls shall be continuous

...ld be one mesh plus two inches at splices.
...cing bars each side around openings in concrete walls and ...
...nd 24" beyond the corners of the openings. Also provide 2 - ...
...ch corner of the opening.
...l not be welded without the approval of the structural engineer.
...bracing steel shall be performed by welders specifically certified ...
Prior to welding the "carbon equivalent" (CE) of steel shall be ...
...ing steel whose "CE" can be identified or whose "CE" exceeds ...
...ished. Except for reinforcing steel conforming to ASTM A-706 ...
...be preheated as shown in table 1, RUA 2-77. In addition, steel ...
0.45% and 0.75% shall be welded only when prior qualification ...
...weldability.

2
...masonry units shall conform to ASTM C90 Grade N Type I, in ...
...xcess of the quality control standards of the Concrete Masonry ...
...a required compressive strength of block units shall be 2100 psi.

...ave been cured for not less than 28 days when placed in the ...

...s will not be permitted in the grout or mortar unless substantiat...
...to and approved by the structural engineer or the architect.
...inforced masonry walls and bond beams shall have f'c = 3,000 ...
...ll have a slump of 8" to 10".
...ll have horizontal reinforcing consisting of galvanized standard ...
...Wall" or Equal. All reinforcing shall be located every other course ...
...
...ll match the size and number of footing reinforcing unless noted ...
...footing 12" minimum and lap 30 diameters with main steel.
...for bearing walls below grade. Type "N" mortar is required for ...
...grade. Mortar shall be of a portland cement type mix.
...s for location and detail of vertical control joints.
...control joints in masonry walls shall not exceed 24'-0" o.c.
...g to masonry shall have two cores minimum filled with grout ...
...ring point, except as noted on the plans.
...all... use one #5 continuous. Bond beams 10" or larger use 2 - ...
...noted otherwise.
...be performed in accordance with ASTM E447-74. Prepare 1 ...
...the start of construction. Test 2 of these at 7 days and the ...
... Prepare 1 additional set of three prisms for every 5000 sf ...
...1 of these at 7 days and the remaining 2 at 28 days. Refer to ...
...to the structural notes and to the project specifications for ...
...

...D TRUSSES: @ 15,000
...d trusses shall be in accordance with manufacturer's written recommendations ...
...have bridging and bracing in accordance with manufacturer's requirements.
...be designed for a top chord live load as noted under Design Live Loads plus ...
...accordance with applicable building codes (minimum top chord dead load shall ...
...bottom chord dead load shall be 10 psf).
...limitations — roof trusses less than L/360.
...op drawings for approval, showing the erection plan, all bearing conditions, ...
...calculations certified by a Professional Engineer will be required for all wood ...
...and bearing attachments/components. (Submit certified plans and calcula...
...tions officially).
...s shall be... and bearing of trusses so as not to exceed perpendicular to grain ...
...ers that support the trusses.

...or foundations shall be pressure treated Southern Pine No. 2.
f. All bolt heads and nuts bearing on wood shall have standard cut washers. All bolt holes in wood shall be drilled 1/32" larger in diameter than nominal bolt diameters.
g. Bolts in wood shall not be less than 7 diameters from the end & 4 diameters from edge of member.
h. All framing anchors, post caps, bases, hangers, straps, etc., shall be as manufactured by "Simpson" Company or approved equal.
i. Top plates of all wood stud walls to be 2 - 2x (same width as studs), lap 48" (min.) with not less than 6-16d nails at each lap and not more than 16" between nails. Splice at studs only.
j. Moisture content of wood at time of placing shall not exceed 19%.
k. All member sizes given on the drawings are nominal sizes.
l. Spacing of bridging for joists shall not exceed 8'-0".
m. Wood lintels and headers shall have a full 3" length of bearing at each end (min.).
n. Double all joists under parallel partitions.
o. All beams and joists not bearing on supporting members shall be framed with "Simpson HUS" joist hangers or equal. Use type as required for the application shown on the drawings.
p. Wood joists shall bear the full width of supporting members (stud walls, beams, etc.) unless otherwise indicated.
q. Wood beams made of 2 or more 2x's shall be nailed together with 2 rows of 16d nails at 12" o.c. For a 3 piece member, install the specified nailing on each side.
r. Unless noted, 4" wide stud walls are to have 2 x 4 studs at 16" o.c.
s. Unless noted, exterior walls are to be 2 x 6 studs at 16" o.c.
t. Sill plates to be bolted to foundation walls with 5/8" round bolts at 4'-0" o.c. maximum, bolts to extend 15" minimum into grouted masonry. Each sill plate to have a minimum of 2 bolts with one bolt located within 12" of each end of each plate.

11. NAILING SCHEDULE:
a. Joists or rafters to sides of studs
3" joist or less 3 - 16d
for each additional 4" in depth of joist 1 - 16d
b. bridging to joists, toenail each end 2 - 8d
blocking between joists or rafters to joists or rafters – toenail each side, each end 2 - 16d
blocking between studs each end 2 - 10d toenails or 2 - 16d
c. 1" x 6" subfloor to each joist, face nail 2 - 8d
d. wider than 1" x 6" to each joist, face nail 3 - 8d
e. 2" subfloor to joist or girder, blind and face nail 2 - 16d
f. Sole plate to joist or blocking, face nail 16d @ 16" o.c.
g. Top plate to stud, end nail 2 - 16d
h. Stud to sole plate 4 - 10d toenail, or 2 - 16d end nail
i. Double top plates, face nail 16d @ 16" o.c.
j. Continuous header, two pieces 16d @ 12" o.c. along each edge
k. Ceiling joists to plate, toenail 3 - 8d
l. Continuous header to studs, toenail 4 - 8d
m. Ceiling joists, laps over partitions, face nail 3 - 16d
n. Ceiling joists to parallel rafters, face nail 3 - 16d
o. Joists or rafters at all bearings, toenail each side 2 - 16d
p. 1" brace to each stud and plate, face nail 2 - 8d
q. 1" x 8" sheathing or less to each bearing, face nail 2 - 8d
r. wider than 1" x 8" sheathing to each bearing, face nail 3 - 8d
s. Built-up corner studs 16d @ 24" o.c.
t. Double studs 10d @ 12" o.c. direct
u. Rim joists 6" or less 16d (2/ joist) end nail
v. Rim joists 8" or more 16d (3/ joist) end nail
w. Diagonal bracing (to stud & plate) 16d (2/ joist) end nail

12. BACKFILLING:
a. No backfilling and compacting of earth shall be permitted against foundation walls until supporting slabs have been poured and have reached 75% of their design strength or unless adequate bracing submitted for review has been approved.
b. Both sides of foundation walls shall be backfilled simultaneously so as to prevent overturning or lateral movement of walls.
c. All grade beams shall be adequately braced to prevent lateral movement during backfilling and compaction.

13. CONSTRUCTION AND CONTROL JOINTS:
a. Construction joints shall be made as detailed on the drawings.
b. Maximum spacing for control joints shall be 15'-0".
c. A 15'-0" maximum spacing of control joints may not insure complete control of shrinkage cracks. A closer spacing may be used by request of the contractor if more complete shrinkage crack control is desired. Contractor to verify with Owner.

14. EXPANSION BOLTS:
a. All expansion bolts shall be KWIK bolts or REDHEAD as noted on the plans. Minimum embedment unless otherwise noted shall be 4" for 1/2" diameter bolts, and 5" for 5/8" and 3/4" diameter bolts.

15. CONSTRUCTION PROCEDURES:
a. The structure shall be adequately braced and shored against wind and construction loads during erection. Structural members are designed for "in-place" loads only.
b. Comply with all applicable city, state and federal laws, including Occupational Safety and Health Administration Act (OSHA) and regulations adopted pursuant thereto.
c. The contract structural drawings and specifications represent the finished structure. Unless otherwise noted, they do not indicate the means or method of construction. Provide all measures necessary to protect the structure, workmen, or other persons during construction. Such measures shall include, but are not limited to, bracing, shoring for construction equipment, shoring for earth loads, forms, scaffolding, planking, safety nets, support and bracing for cranes and gin poles, etc.
d. Engage properly qualified persons to determine where and how temporary precautionary measures shall be used and inspect same in the field. Observation visits to the jobsite by the ENGINEER'S representative shall not include inspection of the above items.
e. Supervise and direct the work so as to maintain sole responsibility for all construction means, methods, techniques, sequences, and procedures. As a part of this responsibility, retain the services of a licensed structural engineer to design and supervise any scaffolding for workmen, and all shoring of forms and elements of the construction.

16. COORDINATION WITH ARCHITECTURAL DRAWINGS:
a. The contractors shall verify all dimensions and elevations with the architectural drawings. Where discrepancies occur, it is the contractors responsibility to notify the Architect prior to proceeding with construction.

17. NEW WORK IN CONJUNCTION WITH EXISTING CONSTRUCTION:
a. The contractors shall verify, by field check, all sizes, dimensions, elevations, locations, etc. of elements of the existing construction which are relative to the new construction.
b. All dimensions involving new work tying into or governed by existing construction shall be field checked by the contractors and furnished to the subcontractors prior to fabrication of any work. The verified dimensions shall appear and be noted as such on the first shop drawings submitted.
c. Cutting of existing structural steel is prohibited without approval of the engineer.

18. SHOP DRAWINGS:
a. Shop drawings, unless otherwise noted, shall be submitted as reproducible transparencies with one print for review prior to fabrication. Send reproducible transparencies and print in a mailing tube. Transparencies which will not produce a legible print shall be returned for resubmittal.
b. Shop drawings are to be prepared under the supervision of a registered professional engineer. They are to include complete details and schedules for fabrication and assembly of structural members, procedures and diagrams.
c. Fabricators shall draw their own erection plans. Copying the structural plans and using them as erection plans is not acceptable.
d. Prior to submittal, the contractors shall review the shop drawings and make any corrections required. The contractors shall stamp and sign the drawings as evidence that they have reviewed them.
e. Shop drawings shall be submitted for all structural components.
f. Turn around time for shop drawings shall be two weeks from the date received in the engineers office.

lamb architects
P.O. Box 310 Forest Lake, Minnesota
(612)

I hereby certify that this plan, specification, or report was prepared by me or under my direct supervision and that I am a duly Licensed Professional Engineer under the laws of the State of Minnesota. [redacted]
Date 9/18/08

1st CHURCH of CHRIST SCIENTIST
CHURCH ROOF RE-DESIGN & REPLACEMENT
ST. PAUL, MINNESOTA

Project No. 0810
Date: 9/18/08
Revisions:

Sheet No. S-1

A. Prefinished metal flashing and keepers (copper to match existing)

B. Prefinished gutters and downspouts (copper to match existing)

C. Existing Brick

D. New Brick (to match existing)

E. New or existing trim – painted (to match existing)

F. Prefinished aluminum louver vents (w/ insect screens)

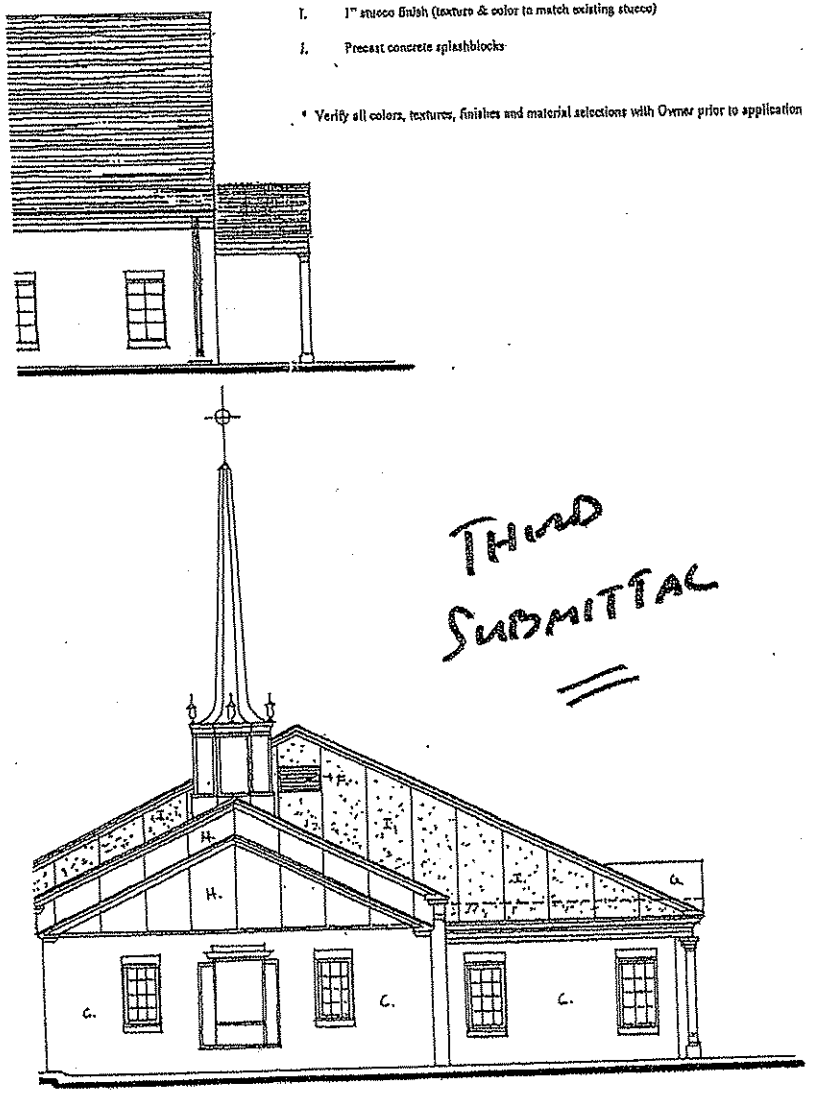
G. 340# fiberglass reinforced asphalt shingles

H. Existing stucco finish

I. 1" stucco finish (texture & color to match existing stucco)

J. Precast concrete splashblocks

* Verify all colors, textures, finishes and material selections with Owner prior to application



THIRD SUBMITTAL

WEST ELEVATION
1/8" = 1'-0"

Jamb architects
P.O. Box 310 Forest Lake, Minnesota (612) 6

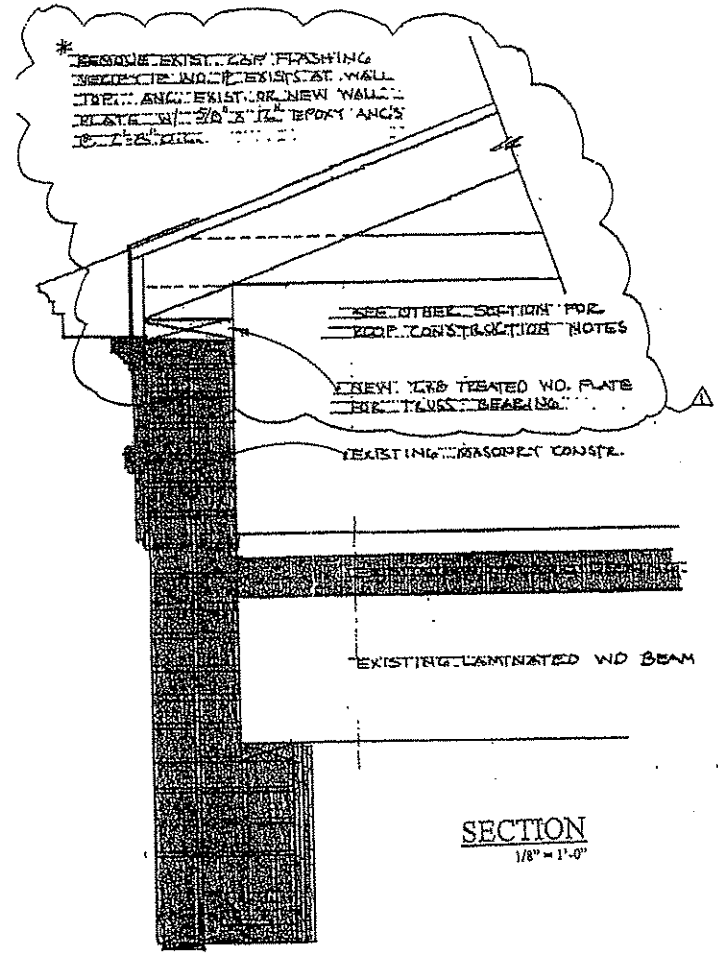
I hereby certify that this plan, specification or report was prepared by me or under my direct supervision and that I am a duly Licensed Architect under the laws of the State of Minnesota.

JEFFREY L. ANDERSON

1st CHURCH of CHRIST SCIENTIST
CHURCH ROOF RE-DESIGN & REPLACEMENT
ST. PAUL MINNESOTA

Project No. 0810
Date 9/18/08
Revisions:

Sheet No. A-1



SECTION
1/8" = 1'-0"

1st CHURCH of CHRIST SCIENTIST

CHURCH ROOF RE-DESIGN & REPLACEMENT
ST. PAUL MINNESOTA

P.O. Box 310 Forest Lake, Minnesota
(612)

Project No. 0810
Date: 9/18/08
Revisions: △ BEARING 6/3/08
Sheet No. A-2

SOIL BEARING [illegible]
[illegible] Building Code w/ State of Minnesota Appendix

[illegible] shall bear on natural undisturbed soil or on compacted granular fill.
[illegible] for subsoiling soils in the footing areas referred to in the soil
[illegible] above shall be strictly followed.
[illegible] bottom of the footings as detailed is of questionable bearing value, the
[illegible] engineers office shall be notified at once.
[illegible] elevation changes shall be stepped at a ratio of 1 (vert) to 2 (horiz).
[illegible] vertical step shall be 1'-6" unless noted otherwise.
[illegible] wall footings shall have a minimum soil cover of 3'-6" measured from
[illegible] of the footing unless otherwise noted.
[illegible] for anticipated settlement values. The structural engineer shall verify
[illegible] criteria will not be detrimental to the building or it's operation.

[illegible], corridors and stairs	100 psf
[illegible]	*10 psf + 30 psf
	125 psf
	**120 psf
	90 mph
	Material Self Weights

[illegible]
weight if heavier
and/or sliding snow

[illegible]

[illegible]	Type Mix		Location
	Std. Weight		Interior slabs and walls
	Std. Weight		Exterior slabs and walls
	Air Entrained		
	Std. Weight		Footings
	Std. Weight		Interior Topping
	f'm = 1,500 psi (28 days)		
[illegible]	Fy = 60,000 psi		ASTM A615
[illegible]	Fy = 36,000 psi		ASTM A36
[illegible]	Fy = 46,000 psi		ASTM A500, Grd. B
[illegible]	Fy = 35,000 psi		ASTM A53, Grd. B
	Fy = 36,000 psi		ASTM A36
	Fu = 120,000 psi		ASTM A325
	Fu = 60,000 psi		ASTM A307
[illegible]	Fy = 70,000 psi		
Fabric			ASTM A185

[illegible] FOR REINFORCEMENT:
3"
2"
Center of Slab
Center of Wall

[illegible] FOR REINFORCEMENT:
Center Line (unless otherwise noted)
3" to dia

[illegible]:
[illegible] steel contractor shall fabricate all reinforcement and furnish all
[illegible], spacer bars and supports necessary to secure the reinforcement
[illegible] otherwise on the plans and details.
[illegible] shall be placed according to the CRSI "Recommended
[illegible] Placing Reinforcing Bars".
[illegible] tension lap splices for cast-in-place concrete shall be 36 bar
[illegible], unless noted otherwise.
[illegible] for reinforced masonry shall be 48 bar diameters minimum,
[illegible] otherwise.
[illegible] steel in footings and concrete walls shall be continuous
[illegible]
[illegible] should be one quarter plus two inches at splices.
[illegible] reinforcing bars each side around openings in concrete walls and
[illegible] extend 24" beyond the corners of the openings. Also provide 2 -
[illegible] at each corner of the opening.
[illegible] shall not be welded without the approval of the structural engineer.
[illegible] of reinforcing steel shall be performed by welders specifically certified
[illegible] steel. Prior to welding the "carbon equivalent" (CE) of steel shall be
[illegible] reinforcing steel whose "CE" can't be identified or whose "CE" exceeds
[illegible] be welded. Except for reinforcing steel conforming to ASTM A-706
[illegible] shall be preheated as shown in table 1.KGA 3-77. In addition, steel
[illegible] 0.45% and 0.75% shall be welded only when prior qualification
[illegible] acceptable weldability.

MASONRY:
[illegible] masonry units shall conform to ASTM C90 Grade N Type 1, in
[illegible] requirements of the quality control standards of the Concrete/Masonry
[illegible] minimum required compressive strength of block units shall be 2100 psi
[illegible])
[illegible] shall have been cured for not less than 28 days when placed in the

[illegible] will not be permitted in the grout or mortar unless substantiated
[illegible] submitted to and approved by the structural engineer or the architect.
[illegible] reinforced masonry walls and bond beams shall have f'c = 3,000
[illegible] and shall have a slump of 8" to 10".
[illegible] shall have horizontal reinforcing consisting of galvanized standard
[illegible]-O-Wall" or equal. All reinforcing shall be located every other course
[illegible] otherwise.
[illegible] shall match the size and number of footing reinforcing unless noted
[illegible] into footing 12" minimum and lap 30 diameters with main steel.
[illegible] for bearing walls below grade. Type "N" mortar is required for
[illegible] grade. Mortar shall be of a portland cement type mix.
[illegible] Plan for location and detail of vertical control joints.
[illegible] for control joints in masonry walls shall not exceed 24'-0" o.c.
[illegible] bearing on masonry shall have two cores minimum filled with grout
[illegible] bearing point, except as noted on the plans.
[illegible] or smaller use one #5 continuous. Bond beams 10" or larger use 2 -
[illegible] unless noted otherwise.
[illegible] shall be performed in accordance with ASTM E447-84. Prepare 3
[illegible] prior to the start of construction. Test 2 of these at 7 days and the
[illegible] 28 days. Prepare 1 additional set of three prisms for every 5000 sf
[illegible] Test 1 of these at 7 days and the remaining 2 at 28 days. Refer to
[illegible]" in the structural notes and to the project specifications for
[illegible].

WOOD TRUSSES:
[illegible] wood trusses shall be in accordance with manufacturer's written recommendations
[illegible] shall have bridging and bracing in accordance with manufacturer's requirements.
[illegible] shall be designed for a top chord live load as noted under Design Live Loads plus
[illegible] in accordance with applicable building codes (minimum top chord dead load shall
[illegible] bottom chord dead load shall be 10 psf.)
[illegible] deflection limitations - roof trusses less than L/360.
[illegible] shop drawings for approval, showing the erection plan, all bearing conditions,
[illegible]. Calculations certified by a Professional Engineer will be required for all wood
[illegible] and bearing attachments/components. (Submit certified plans and calcula-
[illegible] building officials).
[illegible] shall detail end bearing of trusses so as not to exceed perpendicular to grain
[illegible] of plates that support the trusses.

d. All nailing shall conform to the nailing schedule unless noted otherwise.
e. All sills and plates resting on concrete or masonry, which is in contact with the earth or resting on foundations shall be pressure treated Southern Pine No. 2.
f. All bolt heads and nuts bearing on wood shall have standard cut washers. All bolt holes in wood shall be drilled 1/32" larger in diameter than nominal bolt diameters.
g. Bolts in wood shall not be less than 7 diameters from the end & 4 diameters from edge of member.
h. All framing anchors, post caps, bases, hangers, straps, etc., shall be as manufactured by "Simpson" Company or approved equal.
i. Top plates of all wood stud walls to be 2 - 2x (same width as studs), lap 48" (min.) with not less than 6-16d nails at each lap and not more than 16" between nails. Splice at studs only.
j. Moisture content of wood at time of placing shall not exceed 19%.
k. All member sizes given on the drawings are nominal sizes.
l. Spacing of bridging for joists shall not exceed 8'-0".
m. Wood lintels and headers shall have a full 3" length of bearing at each end (min.).
n. Double all joists under parallel partitions.
o. All beams and joists not bearing on supporting members shall be framed with "Simpson HHUS" joist hangers or equal. Use type as required for the application shown on the drawings.
p. Wood joists shall bear the full width of supporting members (stud walls, beams, etc.) unless otherwise indicated.
q. Wood beams made of 2 or more 2x's shall be nailed together with 3 rows of 16d nails at 12" o.c. For a 3 piece member, install the specified nailing on each side.
r. Unless noted, 4" wide stud walls are to have 2 x 4 studs at 16" o.c.
s. Unless noted, exterior walls are to be 2 x 6 studs at 16" o.c.
t. Sill plates to be bolted to foundation walls with 5/8" round bolts at 6'-0" o.c. maximum, bolts to extend 15" minimum into grouted masonry. Each sill plate to have a minimum of 2 bolts with one bolt located within 12" of each end of each plate.

11. NAILING SCHEDULE:
a. Joists or rafters to sides of studs
1" joist or less 3 - 16d
for each additional 4" in depth of joist 1 - 16d
b. bridging to joists, toenail each end 2 - 8d
blocking between joists or rafters to joists or rafters - toenail each side, each end 2 - 10d
blocking between studs each end 2 - 10d toenails or 3 - 16d
c. 1" x 6" subfloor to each joist, face nail 2 - 8d
d. wider than 1" x 6" to each joist, face nail 3 - 8d
e. 2" subfloor to joist or girder, blind and face nail 2 - 16d
f. Sole plate to joist or blocking, face nail 16d @ 16" o.c.
g. Top plate to stud, end nail 2 - 16d
h. Stud to sole plate 4 - 10d toenail, or 2 - 16d end nail
i. Double top plates, face nail 16d @ 16" o.c.
j. Continuous header, two pieces 16d @ 12" o.c. along each edge
k. Ceiling joists to plate, toenail 3 - 8d
l. Continuous header to studs, toenail 4 - 8d
m. Ceiling joists, laps over partitions, face nail 3 - 16d
n. Ceiling joists to parallel rafters, face nail 3 - 16d
o. Joists or rafters at all bearing, toenail each side 2 - 10d
p. 1" brace to each stud and plate, face nail 2 - 8d
q. 1" x 8" sheathing or less to each bearing, face nail 2 - 8d
r. wider than 1" x 8" sheathing to each bearing, face nail 3 - 8d
s. Built-up corner studs 16d @ 24" o.c.
t. Double studs 10d @ 12" o.c. direct
u. Rim joists 6" or less 16d (3/ joist) end nail
v. Rim joists 4" or more 16d (3/ joist) end nail
w. Diagonal bracing (to stud & plate) 16d (3/ joist) end nail

12. BACKFILLING:
a. No backfilling and compacting of earth shall be permitted against foundation walls until supporting slabs have been poured and have reached 75% of their design strength or unless adequate bracing submitted for review has been approved.
b. Both sides of foundation walls shall be backfilled simultaneously so as to prevent overturning or lateral movement of walls.
c. All grade beams shall be adequately braced to prevent lateral movement during backfilling and compaction.

13. CONSTRUCTION AND CONTROL JOINTS:
a. Construction joints shall be made as detailed on the drawings.
b. Maximum spacing for control joints shall be 15'-0".
c. A 15'-0" maximum spacing of control joints may not insure complete control of shrinkage cracks. A closer spacing may be used by request of the contractor if more complete shrinkage crack control is desired. Contractor to verify with Owner.

14. EXPANSION BOLTS:
a. All expansion bolts shall be KWIK bolts or REDHEAD as noted on the plans. Minimum embedment unless otherwise noted shall be 4" for 1/2" diameter bolts, and 5" for 5/8" and 3/4" diameter bolts.

15. CONSTRUCTION PROCEDURES:
a. The structure shall be adequately braced and shored against wind and construction loads during erection. Structural members are designed for "in-place" loads only.
b. Comply with all applicable city, state and federal laws, including Occupational Safety and Health Administration Act (OSHA) and regulations adopted pursuant thereto.
c. The contract structural drawings and specifications represent the finished structure. Unless otherwise noted, they do not indicate the means or method of construction. Provide all measures necessary to protect the structure, workmen, or other persons during construction. Such measures shall include, but are not limited to, bracing, shoring for construction equipment, shoring for earth loads, forms, scaffolding, planking, safety nets, support and bracing for cranes and gin poles, etc.
d. Engage properly qualified persons to determine where and how temporary precautionary measures shall be used and inspect same in the field. Observation visits to the jobsite by the ENGINEERS representative shall not include inspection of the above items.
e. Supervise and direct the work so as to maintain sole responsibility for all construction means, methods, techniques, sequences, and procedures. As a part of this responsibility, retain the services of a licensed structural engineer to design and supervise any scaffolding for workmen, and all shoring of forms and elements of the construction.

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a. The contractor shall verify all dimensions and elevations with the architectural drawings. Where discrepancies occur, it is the contractors responsibility to notify the Architect prior to proceeding with construction.

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a. The contractors shall verify, by field check, all sizes, dimensions, elevations, locations, etc. of elements of the existing construction which are relative to the new construction.
b. All dimensions involving new work tying into or governed by existing construction shall be field checked by the contractors and furnished to the subcontractors prior to fabrication of any work. The verified dimensions shall appear and be noted as such on the first shop drawings submitted.
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b. Shop drawings are to be prepared under the supervision of a registered professional engineer. They are to include complete details and schedules for fabrication and assembly of structural members, procedures and diagrams.
c. Fabricators shall draw their own erection plans. Copying the structural plans and using them as erection plans is not acceptable.
d. Prior to submittal, the contractor shall review the shop drawings and make any corrections required. The contractor shall stamp and sign the drawings as evidence that they have reviewed them.
e. Shop drawings shall be submitted for all structural components.
f. Turn around time for shop drawings shall be two weeks from the date received by the

Lamb architec[illegible]
P.O. Box 310 Forest Lake, Minnesota [illegible]
(612) 670-[illegible]

I hereby certify that this plan, specification, or report was prepared by me or under my direct supervision and that I am a duly Registered Professional Engineer under the laws of the State of [illegible]
[redacted]
Date 9/18/08 Reg. No. [illegible]

1st CHURCH of CHRIST SCIENTIST
CHURCH ROOF RE-DESIGN & REPLACEMENT
ST. PAUL MINNESOTA

Project No. 0810
Date: 9/18/08
Revisions:

Sheet No. S-1

July 24, 2009

JUL 2 8 2009

The Minnesota State Board of AELSLAG & ID
85 East 7th Place, Suite 160
Saint Paul, Minnesota 55101

Attention: Ms Lynette DuFresne
Reference: File No. 2009-0029

Dear Ms. DuFresne:

Please find the enclosed answers to your request for information in the same order it appears in your letter.

a. Yes, I did draw the plan. As has been our relationship for over twenty years of professional practice, [redacted] conducts the engineering and I act as his draftsman in preparing the work under his direction for his review and eventual certification. It should be noted that [redacted] checked the loading for the existing footings on this particular project as it relates to span of the trusses, I prepared a framing plan for the purpose of obtaining bids, and the final design, certification and calculations are to be provided by the truss manufacturer as specified in section 9. PRE-ENGINEERED WOOD TRUSSES.

b. The plans were prepared on September 18, 2008. They were picked up by the Owner's representative, Mr. Tim Tacheny for the Churches review and comment. I was headed out of town for 12 days and Mr. Tacheny knew the final Engineering work was not complete. [redacted] was to complete this work while I was gone so that we could finalize the project on my return. [redacted] ran the calculations during my absence, I cannot tell you the exact date, but it was between 9/19/09 and 10/02/09. He informed me that Mr. Tacheny had picked up copies of his calculations, etc. while I was gone as he was in a hurry for the building permit.

c. [redacted] did not draw the plans for the 1st Church of Christ Scientist, sheet S-1. The framing plan was discussed with [redacted] and prepared for [redacted] review and approval by Jerry W. Anderson. [redacted] and I have always agreed that he would do the engineering work and I would draft the plans for his review and approval. Please see item a.

d. [redacted] did eventually sign and certify the work for the Church. It was my understanding that that his original calculations were either misplaced or inadvertently taken with the original drawings by Mr. Tacheny while I was out of town. Once Mr. Berg filed the complaint, I tried for some time to help [redacted] retrieve the original Church plans so he could recalculate the loads for the existing footings. This process was somewhat complicated by the fact that our contact at the Church had been removed from the Project. I finally contacted Mrs. Kathy Furch of

EXHIBIT E

d. the Church to again obtain copies of the original plans for [redacted]use. It was now the middle of February and [redacted]again ran calculations for the truss loads on the existing footings. I know that [redacted]signed and forwarded copies to Mr. Frank Berg and had him copy me for my files. The original signature on sheet S-1 was a sticker given to me some time ago by [redacted]. I assume that the signature on his 02/21/09 calculations is original.

e. Mr. Tacheny was supposed to forward copies of the work to [redacted]for his engineering input, review and calculations while I was out of town. My understanding is that Mr. Tacheny attempted to obtain a building permit without the Engineering work being completed. I provided Mr. Tacheny with two separate proposals for the work at the Church. He gave me the go ahead for the drawing phase without the use of field as-built verification or design work. I had informed Mr. Tacheny that final engineering would be required to obtain a permit and that the work should be completed during my absence. I spoke with [redacted]regarding this work and he indicated that he could have it finished before my return. However, Mr. Tacheny turned the plans into the City prior to obtaining the final engineering review. I placed [redacted] signature (sticker) on the drawing anticipating that he would be reviewing the final project details and framing plan as well as confirming his load calculations.

f. Yes, we agreed that I would prepare the drawing and he would review and prepare calculations for the existing footing loads. I had also discussed with him the load placement for the trusses and whether the existing roof could be left in-place to help keep the Church weather-tight during the construction. [redacted]told me that Mr. Tacheny did come to his office while I was out of town and that he reviewed the drawings and prepared calculations for him. I met with [redacted] upon my return, reviewed the work with him and prepared detail changes he requested (removal of the knee wall for bearing and bearing on plates attached to the top of the existing masonry wall - see the revised plans dated 10/03/08). He did not require any revisions to the S-1 sheet at that time and he thought Mr. Tacheny had forwarded his calculations to Mr. Frank Berg.

g. We did not agree that I would prepare Structural Design Plans on the Church or any other projects. We had an understanding that [redacted] would perform all engineering work and that I would draw the plans and details for him. It was not our normal practice to use stickers for signature plates. [redacted]had given me a few to use for him when timing issues arose or when he might be out of town and deadlines needed to be maintained. He always reviewed his work and provided input to me as to the final engineering work.

h. [redacted] was compensated with a + or - $250.00 payment in the nature of Services traded. Because of his health he has limited mobility and is unable to easily visit jobsites. I made a trip to one of [redacted] projects to prepare field measurements and as builts for him. My time equated very closely to his costs on the Church so we simply traded those Services.

[redacted] has always been professional and thorough in our working relationship. It is my feeling that two things happened which led to this situation.

1) The fact that I was traveling did not allow for proper communications
2) The fact that Mr. Tacheny attempted to forgo the cost of Engineering or attempted to obtain a building permit without it being complete creates the appearance of wrong doing when in fact that is not the case.

If you should have any further questions or require additional information or clarification, please do not hesitate to contact me.

Respectfully Submitted,

Jerry W. Anderson

AFFIDAVIT OF SERVICE BY MAIL

RE: In the matter of Jerry Wayne Anderson,
ARCHITECT
License Number 13639

STATE OF MINNESOTA)
) ss.
COUNTY OF RAMSEY)

Lynette DuFresne, being first duly sworn, deposes and says:

That at the City of St. Paul, County of Ramsey and State of Minnesota, on this the 8th day of FEBRUARY, 2010, she served the attached Stipulation and Order by depositing in the United States mail at said city and state, a true and correct copy thereof, properly enveloped, with first class and certified postage prepaid, and addressed to:

Mr. Jerry Wayne Anderson
Jamb Architects
Post Office Box 310
Forest Lake, Minnesota 55025

CERTIFIED MAIL
RETURN RECEIPT REQUESTED
Number 7003 3110 0004 8527 5477

Lynette M. DuFresne
Lynette DuFresne

Subscribed and sworn to before me on
this the 8th day of February, 2010.

Lisa Maria DeTomaso
(Notary Public)

LISA MARIA DETOMASO
NOTARY PUBLIC - MINNESOTA
MY COMMISSION
EXPIRES JAN. 31, 2011